National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Interim Advice Note

on

Road Link Design

for

2+1 Roads

Revision 'A' - February 2006

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Summary:

This Interim Advice Note sets out the elements of design for use in the geometric design of roads, including the new 2+1 road type. It is modelled on TD 9, with the elements for 2+1 added. Design standards for 2+1 roads are also given, including details of cross-sections and junctions.

This document will not be issued as an NRA DMRB standard until monitoring of the 2+1 pilot projects has been completed.

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ROAD LINK DESIGN

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0. FOREWORD

Introduction

0.1 This Interim Advice Note applies to the new 2+1 road type. Design standards for both single and dual carriageway roads in both urban and rural areas have been reproduced within this advice note for convenience. The Interim Advice Note shall be used to derive the Design Speed, and the appropriate values of geometric parameters for use in the design of the road alignment. It states the basic principles to be used for coordinating the various elements of the road layout, which together form the three dimensional design of the road.

0.1A This Interim Advice Note sets out the elements of design for use in the geometric design of roads, including the new 2+1 road type. It is modelled on TD 9, with the elements for 2+1 added. Design standards for 2+1 roads are also given, including details of cross-sections and junctions.

The principal changes from the previous Interim Advice Note are:

a) All reference to TD9 removed to avoid confusion with published TD9.

b) Fig 9/4(a) revised to remove dimensions in median.

c) Fig. 9/4(b) added - Cross-Section Widths for '2+1 Roads' with Overlapping Passing Lanes at Restricted Locations with shared pedestrian/cycleway facilities.

d) More information added under 'Vehicular access' heading, Paragraph 9.35.

e) Pedestrian and Cyclists requirements added , Paragraphs 9.38, 9.39 and Fig 9/13c.

f) Subsequent Paragraphs renumbered.

g) Safety Barrier detail at junction added, Paragraph 9.45 and Annex D.

h) Note on reflectorised disc added, Paragraph 9.53 and Annex D.

i) Note on Monitoring added, Paragraph 9.54 and Annex B.

j) Note on Maintenance added, Paragraph 9.55 and Annex C

k) Recommended layouts for 2+1 junctions are contained in Annex D.

l) Paragraph 1.31A added – although this is an IAN relaxations and Departures shall apply.

0.2

Definitions

0.3 For the definitions of the general road terms used in this Interim Advice Note, such as components of the road (central reserve, verge, hard shoulder and hard strip, etc.), see BS 6100: Subsection 2.4.1.

0.4 Particular terms used in this Interim Advice Note are defined as follows:

All purpose road:- A road for the use of all classes of traffic (e.g. not a motorway).

Central reserve:- The area which separates the carriageways of a dual carriageway road or a 2+1 road. Note that this <u>includes</u> any offside hard strips.

D2AP or D3AP:- Dual two-lane (or dual three-lane) all-purpose road.

D2M or D3M:- Dual two-lane (or dual three-lane) motorway.

Hard Shoulder:- Surfaced strip, greater than 1.5m wide, adjacent to a carriageway intended for use by vehicles in the event of a difficulty or during obstruction of the carriageway. A hard shoulder does not form part of the verge.

Hard Strip:- Surfaced strip, not more than 1.5m wide, that abuts a carriageway. A hard strip forms part of the verge.

High Quality Dual Carriageway:- Dual two-lane (or dual three-lane) all-purpose road constructed to the geometric standards of NRA TD 9 and TD 22 for a motorway.

Roads: Urban and Rural: An **Urban Road** is a road which is in a built-up area and has either a single carriageway with a speed limit of 40mph or less, or has a dual carriageway (including motorways) with a speed limit of 50mph or less. All other roads are **Rural Roads.**

S2:- Two-lane single carriageway road with lane widths of up to 3.75m.

Verge:- The part of a road cross-section alongside a carriageway but not including embankment or cutting slopes. Note that this includes hard strips but not hard shoulders.

WS2:- Two-lane wide single carriageway, normally with lane widths of 5.0m.

2+1 Road:- A road with two lanes in one direction and one in the other. The two-lane section is provided alternately for each direction of travel. A 2+1 is a 'divided' road, with a central reserve marked either by road markings and a safety barrier or delineator posts or by road markings alone.

0.5 For the purposes of this Interim Advice Note, a 2+1 road is considered as a separate category of road – neither a single carriageway, nor a dual carriageway. Thus, references to a single carriageway, or to a dual carriageway, do not include a 2+1 road. A single carriageway road with a climbing lane (see Chapter 5) is not classed as a '2+1 road'.

0.6 (Not used)

0.7 The principal design parameters for the layout of road links are based on "Desirable Minimum" values. Values of parameters below the Desirable Minimum are expressed in terms of the number of Design Speed steps below the Desirable Minimum. However, some other DMRB Standards refer to Absolute Minimum values of parameters in this Standard. Where this occurs, the reference shall be taken to mean one Design Speed step below the Desirable Minimum value.

Implementation

0.8 This Interim Advice Note shall be used for the design of all new or improved national 2+1 roads. Unless otherwise agreed with the relevant Road Authority, it shall also be used on national road schemes for the design of all roads with a Design Speed of 50km/h or more. The Interim Advice Note should be applied to the design of schemes already being prepared unless, in the opinion of the National Roads Authority, application would result in significant additional expense or delay progress. In such cases, Design Organisations should confirm the application of this Interim Advice Note to particular schemes with the National Roads Authority.

0.9 If this Interim Advice Note is to be used for the design of local road schemes, the designer should agree with the relevant Road Authority the extent to which the document is appropriate in any particular situation.

Scope

0.10 A major objective of this Interim Advice Note is to ensure that designs achieve value for money without any significant effect on safety. The design systems that have been developed in relation to both Design Speed and the related geometric parameters will result in greater flexibility to achieve economic design in difficult circumstances. In addition, detailed attention is given to the design of single carriageway roads, where the recommendations allow flexibility for design, with particular emphasis upon the coordination of design elements to improve safety and overtaking conditions. Overall, the flexibility for design introduced by this Interim Advice Note will enable economic designs to be prepared, minimising both the construction costs and the impact of new roads and road improvements on the environment.

0.11 Throughout this Interim Advice Note, there are continual references to the use of cost/benefit analyses. These should be used at all stages to test the economic performance of alternative scheme designs.

Interpretation

0.12 The standards contained in this document represent the various criteria and maximum/minimum levels of provision whose incorporation in the road design would achieve a desirable level of performance in average conditions in terms of traffic safety, operation, economic and environmental effects. In most cases, with care, designs can be achieved which do not utilise the lowest levels of design parameters given. At some locations on new roads or major improvements, however, it may not be possible to justify even the lowest levels of design parameters in economic or environmental terms, due to high costs, low traffic levels, and environmental damage, etc. In such cases, sufficient advantages might justify either a Relaxation within the standards or, in more constrained locations, a Departure from the standards. The various parameters quoted in this Interim Advice Note are not, therefore to be regarded as sacrosanct in all circumstances. Relaxations and Departures should be assessed in terms of their effects on the economic worth of the scheme, the environment, and the safety of the road user. Further details on the use of Relaxations are given in Chapters 1 to 4.

0.13 Designers should always have regard to the cost effectiveness of the design provision. However, the implications, particularly in relation to safety may not be quantifiable and the designer must apply the judgement of experience in proposing a Relaxation or Departure.

0.14 When issued in the United Kingdom in 1981, this Standard introduced the concept of a hierarchy of permitted values for geometric layout parameters (visibility, horizontal curvature and vertical curvature). This hierarchy was based upon Desirable Minimum standards, with lower values being known progressively as Relaxations and Departures. Values equal to or higher than Desirable Minimum give consistently safe alignments and minimise journey times. However, research had shown that in many situations safety was no worse with values lower than the rigid requirements of the previous standards. The hierarchy of values enabled a flexible approach to be applied where the strict application of Desirable Minimum requirements would lead to disproportionately high construction costs or severe environmental impacts upon people, properties and landscapes. Successive levels in the hierarchy invoked more stringent consideration in line with the need to consider safety carefully.

0.15 During the years since 1981 there have been many advances in road layout design. The procedures for the assessment of safety and operational aspects have improved. Further research has strengthened the understanding of driver behaviour. Safety audits and other initiatives in the mechanics of assessing and checking scheme layouts have made the design process more rigorous and reliable.

0.16 Since 1981, experience has been gained in the application of this hierarchy of values and this indicates that the environmental and financial benefits gained from increased flexibility can be considerable. Against this background, the scope for Relaxations has been set so as to allow designers to consider alignment parameter values that would generally be approved if they were put to the National Roads Authority as Departure proposals. The designer is required to consider carefully the benefits and any potential disadvantages of Relaxations. Guidance is included in Chapter 1, describing the approach to be taken to assessing Relaxations. Relaxations are considered to conform to standards.

1. DESIGN SPEED

General

1.1 The road alignment shall be designed so as to ensure that standards of curvature, visibility, superelevation, etc. are provided for a Design Speed which shall be consistent with the anticipated vehicle speeds on the road. A relatively straight alignment in flat country will generate higher speeds, and thus produce a higher Design Speed, than a more sinuous alignment in hilly terrain or amongst dense land use constraints. There is, therefore, always an inherent economic trade-off between the construction and environmental costs of alternative alignments of different Design Speeds, and their user benefits.

Factors Affecting Speed

1.2 Speeds vary according to the impression of constraint that the road alignment and layout impart to the driver. This constraint can be measured by the three factors given in Paragraphs 1.3 to 1.5.

1.3 Alignment Constraint, Ac: This measures the degree of constraint imparted by the road alignment, and is measured by:

Dual Carriageways and 2+1 Roads:

$$Ac = 6.6 + B/10$$

Single Carriageways: $Ac = 12 - VISI/60 + 2B/45$

where:

B = Bendiness (total angle the road turns through), degrees/km;

VISI = Harmonic Mean Visibility, m (see Annex A).

1.4 Layout Constraint, Lc: This measures the degree of constraint imparted by the road cross section, verge width and frequency of junctions and accesses. Table 1 shows the values of Lc relative to cross section features and density of access, expressed as the total number of junctions, laybys and direct accesses (other than single field accesses) per km (see TD 41), summed for both sides of the road, where:

L = Low Access numbering up to 5 per km;

M = Medium Access numbering 6 to 8 per km;

H = High Access numbering 9 or more per km.

Table 1: Layout Constraint, Lc km/h

Road Type	S2						WS2		2+1		D2AP				D3AP	D2M		D3M
Carriageway Width (ex. hard strips)	6m		7.0m		7.3m		10m		7.0 + 3.5m		Dual 7.0m		Dual 7.5m		Dual 10.5m or 11.25m	Dual 7.0m	Dual 7.5m	Dual 10.5m or 11.25m
Degree of Access and Junctions	H	M	M	H	M	L	M	L	M	L	M	L	M	L	L	L	L	L
With hard shoulders					21	19	17	15			10	9	8	7	5	5	4	0
Without hard shoulders:																		
With 3.0m Verge	(29)	(26)	25	23	(23)	(21)	(19)	(17)	20	18	(12)	(11)	(10)	(9)	(6)			
With 1.5m Verge	(31)	(28)		(27)			(25)	(23)										
With 0.5m Verge	(33)	(30)																

( ) : Non-standard cross-section

There are no research data available for 4 lane Single Carriageway roads between 12 and 15m width. In the limited circumstances for their use described in this document, Design Speed should be estimated assuming a normal D2AP with a Layout Constraint of 15 - 13km/h.

1.5 Mandatory Speed Limits: On rural derestricted roads, i.e. with national speed limits of:

	km/h
Motorways	120
National Roads (Single and Dual Carriageways and 2+1 Roads)	100
Non-national Roads	80

vehicle speeds are constrained only by the physical impression of the road alignment, as described by Ac and Lc. The use of mandatory speed limits (together with more confined urban cross-sections), however, restricts speeds below those freely achievable, and will act as a further constraint on speed in addition to that indicated by Lc.

Selection of Design Speed

1.6 New Rural Roads: Design Speed shall be derived from Figure 1, which shows the variation in speeds for a given Lc against Ac. The Design Speeds are arranged in bands, i.e. 120, 100, 85 km/h etc., within which suffixes A and B indicate the higher and lower categories of each band.

1.6A An initial alignment to a trial Design Speed should be drawn up, and Ac measured for each section of the route demonstrating significant changes thereof, over a minimum length of 2 km. The Design Speed calculated from the ensuing Ac and Lc should be checked against the initial choice, to identify locations where elements of the initial trial alignment may be relaxed to achieve cost or environmental savings, or conversely where the design should be upgraded, according to the calculated Design Speed. If any changes to road geometry result, then the Design Speed should be recalculated to check that it has not changed.

1.6B The Design Speed calculated in accordance with the above procedure may be greater than the mandatory speed limit for the road. In such cases, the following rules shall apply:

a) On motorways and dual carriageways with a speed limit of 100km/h or greater, the Design Speed shall be as calculated;

b) On single carriageways and 2+1 roads with a speed limit of 80km/h or greater, the Design Speed should be as calculated or 100km/h, whichever is the lesser;

c) On other roads (i.e. those with speed limits less than indicated in Paragraphs a and b for the relevant road type), the Design Speed should be as calculated but need not be greater than the Design Speed indicated in Table 2 for the relevant speed limit.

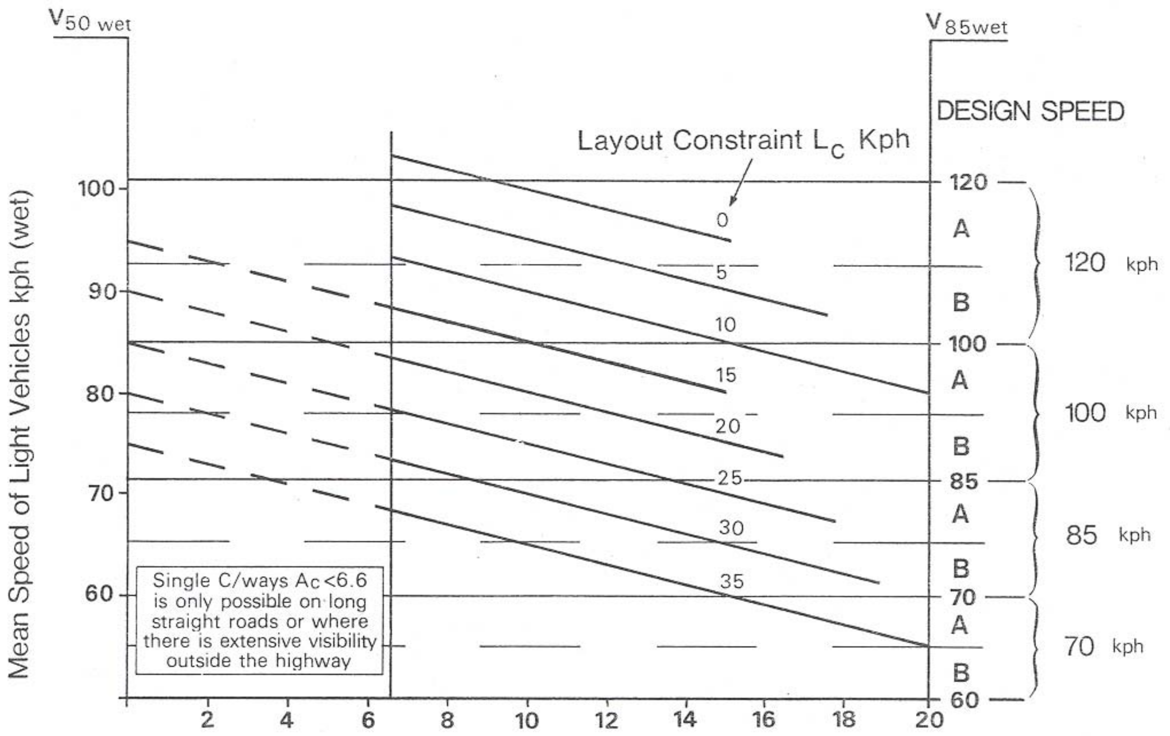
1.6C However, where a proposed layout has isolated sub-standard features, the imposition of a mandatory speed limit (where one would otherwise not be needed) should not be used to justify those features: Departures from Standard should be sought instead (see Paragraph 1.31).

1.7 Existing Rural Road Improvements (including short diversions or bypasses up to about 2 km in length): Design Speed shall be derived in a similar manner to Paragraphs 1.6 to 1.6B above, with Ac measured over a minimum length of 2 km incorporating the improvement, provided there are no discontinuities such as roundabouts. The strategy for the contiguous sections of road, however, must be considered when determining Ac and the cross-sectional design. It might be unnecessary to provide a full standard cross-section for a minor re-alignment within a low standard route, unless it represented an initial stage of a realistic improvement strategy.

1.8 Urban Roads: Low speed limits (30-50 km/h) may be required due to the amount of frontage activity, but also where physical restrictions on the alignment make it impractical to achieve geometry relative to a higher Design Speed. Design Speeds shall be selected with reference to the speed limits envisaged for the road, so as to permit a small margin for speeds in excess of the speed limit, as shown in Table 2.

Table 2: Maximum Design Speeds for Mandatory Speed Limits

Speed Limit km/h	Design Speed km/h
30	50B
50	60B
80	85A



ALIGNMENT CONSTRAINT A_C km/h for Dual C/ways and 2+1 Roads = 6.6 + B/10
for Single C/ways = 12 – VISI/60 + 2B/45

Figure 1: Selection of Design Speed (Rural Roads)

Design Speed Related Parameters

1.9 The Design Speed bands 120, 100, 85 km/h etc. dictate the minimum geometric parameters for the design according to Table 3. This shows Desirable Minimum values and values for certain Design Speed steps below Desirable Minimum. Desirable Minimum values represent the comfortable values dictated by the Design Speed.

Table 3: Design Speed Related Parameters

DESIGN SPEED (km/h)	120	100	85	70	60	50	V^2/R
STOPPING SIGHT DISTANCE m							
Desirable Minimum Stopping Sight Distance	295	215	160	120	90	70	
One Step below Desirable Minimum	215	160	120	90	70	50	
Two Steps below Desirable Minimum	160	120	90	70	50	50	
HORIZONTAL CURVATURE m							
Minimum R* without elimination of Adverse Camber and Transitions	2880	2040	1440	1020	720	510	5
Minimum R* with Superelevation of 2.5%	2040	1440	1020	720	510	360	7.07
Minimum R with Superelevation of 3.5%	1440	1020	720	510	360	255	10
Desirable Minimum R with Superelevation of 5%	1020	720	510	360	255	180	14.14
One Step below Desirable Min R with Superelevation of 7%	720	510	360	255	180	127	20
Two Steps below Desirable Min R with Superelevation of 7%	510	360	255	180	127	90	28.28
VERTICAL CURVATURE – CREST							
Desirable Minimum Crest K Value	182	100	55	30	17	10	
One Step below Desirable Min Crest K Value	100	55	30	17	10	6.5	
Two Steps below Desirable Min Crest K Value	55	30	17	10	6.5	6.5	
VERTICAL CURVATURE – SAG							
Desirable Minimum Sag K Value	53	37	26	20	13	9	
One Step below Desirable Min Sag K Value	37	26	20	13	9	6.5	
Two Steps below Desirable Min Sag K Value	26	20	13	9	6.5	6.5	
OVERTAKING SIGHT DISTANCES							
Full Overtaking Sight Distance FOSD m.	N/A	580	490	410	345	290	
FOSD Overtaking Crest K Value	N/A	400	285	200	142	100	

Notes

* Not to be used in the design of single carriageways (see Paragraphs 7.25 to 7.30).

The V2/R values simply represent a convenient means of identifying the relative levels of design parameters, irrespective of Design Speed.

K Value = curve length divided by algebraic change of gradient (%). See Paragraph 4.5.

Changeover of Design Speed Standards

1.10 Transitions between sections with different Design Speeds shall be designed carefully so as not to present the driver suddenly with low radius curves, shorter sight distances etc. Where an alignment changes from a higher to a lower Design Speed, Relaxations should be avoided adjacent to the interface on the length of road with the lower Design Speed.

Connection to Existing Roads

1.11 Care shall be taken where an improved section rejoins an existing road, that the existing standard of curvature and sight distance at the interface shall be subject to the same restrictions as would be relevant for the Design Speed of the improvement. Figure 2 shows the connection of an improvement to an existing road. Care must be taken that the curvature and sight distance at C is adequate for the approach Design Speed which has increased due to the improvement between A and B.

Figure 2: Connection to Existing Road

Selection of Parameter Values

1.12 Designers should normally aim to achieve at least Desirable Minimum values for stopping sight distance, horizontal curvature and vertical curvature. For single carriageways there are certain horizontal and vertical curve values which, although exceeding the Desirable Minimum values, are not recommended: in some cases Departures from Standards would be required. See Paragraphs 7.25 to 7.31 inclusive.

1.13 Numerous accident studies have been carried out and it has always proved difficult to correlate accident rates with causal factors. The reason is that an accident is a rare, random event where people have failed to cope with the situation; often exacerbated by one or more influences from a large selection of contributory factors. Serious injury accidents are rarer still, with the majority being primarily attributable to driver error. It is estimated that road layout is a main contributory factor in only a small proportion of injury accidents, indicating that accident rates are unlikely to be significantly affected by small or even moderate reductions in design parameters.

1.14 Studies carried out on rural all-purpose roads in the United Kingdom for the development of the UK version of this Standard aimed to correlate personal injury accident rates with horizontal curvature, gradient, and sight distance. Results were consistent with those of other studies, showing that values of these layout parameters below Desirable Minimum values were associated with slightly increased accident rates, but that the increase did not become significant until the difference from the Desirable value was considerable.

Relaxations

1.15 This Standard defines a sequence of parameter values in the form of a hierarchy of geometric design criteria related to Design Speeds. This three tier hierarchy enables a flexible approach to be applied to a range of situations where the strict application of Desirable Minimum standards would lead to disproportionately high construction costs or severe environmental impacts upon people, properties or landscapes. Designs with at least Desirable Minimum standards will produce a high standard of road safety and should be the initial objective. However, the level of service may remain generally satisfactory and a road may not become unsafe where these values are reduced. This second tier of the hierarchy is termed a Relaxation.

1.16 The limit for Relaxations is defined by a given number of Design Speed steps below the Desirable Minimum. Relaxations vary according to the type of road - motorway or all-purpose - and whether the Design Speed is band A or band B. Details for sight distance are given in Chapter 2, for horizontal alignment in Chapter 3, and for vertical alignment in Chapter 4.

1.17 Relaxations may be introduced at the discretion of the designer, having regard to the advice given in this document and all the relevant local factors. Careful consideration must be given to layout options incorporating Relaxations, having weighed the benefits and any potential disbenefits. Particular attention should be given to the safety aspects and the environmental and/or cost benefits which would result from the use of Relaxations. The design organisation shall record the fact that a Relaxation has been used and the corresponding reason for its use. The record shall be endorsed by the design organisation's senior engineer responsible for the scheme. The design organisation shall report all Relaxations incorporated into the design as part of the project report at the end of each project management phase (refer to the National Roads Project Management Guidelines). The preferred option should be compared against options that would meet Desirable Minimum standards.

1.18 A number of layout options might be feasible for a scheme, with each containing Relaxations. This Standard gives examples of locations where some options can be expected to be safer than others. For example, Desirable Minimum Stopping Sight Distance could be provided to a junction, at the expense of a Relaxation to less than desirable values of horizontal or vertical curvature at a location away from that junction. The Relaxation then becomes isolated in that only one feature is below desirable value on a given length of road, and that length does not contain the complication of a junction. In this manner the accident potential of a constrained alignment has been minimised by applying layout design principles based upon the knowledge currently available.

1.19 A list of principles to follow when preparing options that include Relaxations is as follows. It is equally a list of factors to be taken into account when considering the merits of options.

1.20 The designer should consider whether, and to what degree, the site of the proposed Relaxation is:

- isolated from other Relaxations;
- isolated from junctions;
- one where drivers have Desirable Minimum Stopping Sight Distance;
- subject to momentary visibility impairment only;
- one that would affect only a small proportion of the traffic;
- on straightforward geometry readily understandable to drivers;
- on a road with no frontage access;
- one where traffic speeds would be reduced locally due to adjacent road geometry (e.g. uphill sections, approaching roundabouts and major/minor junctions where traffic has to yield or stop etc.), or speed limits.

1.21 The designer should also consider whether the following should be introduced in conjunction with any Relaxation:

- accident prevention or mitigation measures (e.g. increased skidding resistance, safety barriers, etc.);
- warning signs and road markings to alert the driver to the layout ahead.

1.22 The designer should have regard to the traffic flows carried by the link. High flows may carry a greater risk of queues and standing traffic approaching junctions in the peak period. Conversely lower flows might encourage higher speeds.

1.23 Values for sight distance, horizontal curvature and vertical curvature shall not be less than those given in Table 3 for each Design Speed and the appropriate number of Design Speed steps.

1.24 Of the alignment standards, only Stopping Sight Distance, horizontal curvature, vertical curvature, superelevation and gradient shall be subject to Relaxations.

1.25 At any one location, combinations of Relaxations of the alignment standards set out in Chapters 1 to 5 of NRA TD 9 are not permitted except in the following circumstances:

a) Stopping Sight Distance Relaxations of up to one Design Speed step below Desirable Minimum may be coincident with horizontal curvature Relaxations of up to one Design Speed step below Desirable Minimum.

b) The use of a crest curve K value of one Design Speed step below Desirable Minimum to avoid dubious overtaking conditions on a straight or nearly straight section of single carriageway in accordance with Paragraph 7.30, is not regarded as a Relaxation. Such a curve will generally result in a one step Relaxation of Stopping Sight Distance. This arrangement is permitted.

c) A vertical curve K value of up to one Design Speed step below Desirable Minimum may be used at the end of a steep gradient with a permitted Relaxation. However, there shall be no Relaxation in the Stopping Sight Distance at such locations, except as permitted by Paragraph 1.25(b).

d) Stopping Sight Distance Relaxations to the low object at central reserve safety barriers (see Paragraph 2.7A) may be coincident with other Relaxations, provided Desirable Minimum Stopping Distance is obtained to a 1.05m high object.

e) A Relaxation (or permitted combination of Relaxations) of one of the geometric parameters in NRA TD 9 is permitted in combination with a Relaxation from another current design standard in the NRA Design Manual for Roads and Bridges.

No other combinations of Relaxations are permitted. If used, they shall be treated as Departures.

1.26 A crest curve K value Relaxation of one Design Speed step below Desirable Minimum will generally result in a reduction in Stopping Sight Distance to a value one Design Speed step below Desirable Minimum, the adoption of which would also require a Relaxation. With the exception of the case described in Paragraph 1.25(b), this is not a permitted combination of Relaxations and shall be treated as a Departure.

1.27 Relaxations are not permitted for either of the overtaking sight distance parameters given in Table 3.

1.28 The following Relaxations are **NOT** permitted on the immediate approaches to junctions, because the majority of accidents occur in the vicinity of junctions:

a) Relaxations below Desirable Minimum Stopping Sight Distance other than Relaxations to the low object at central reserve safety barriers (see Paragraphs 2.7A to 2.13);

b) Relaxations below Desirable Minimum in vertical curvature for crest curves (see Paragraphs 4.9 to 4.13). This requirement takes precedence over the requirements of Paragraphs 7.19 and 7.30;

c) Relaxations more than one Design Speed step below Desirable Minimum for sag curves (see Paragraphs 4.14 to 4.17).

1.29 For the purposes of this Standard the immediate approaches to a junction shall be:

a) For at grade major/minor junctions without diverge and merge tapers, those lengths of carriageway on the minor roads between a point 1.5 times the Desirable Minimum stopping sight distance upstream of the Stop line or Yield line and the Stop line or Yield line itself, and those lengths of carriageway on the mainline between a point 1.5 times the Desirable Minimum Stopping Sight Distance from the centre line of the minor road and the centre line itself;

b) For roundabouts, those lengths of carriageway on the approach to the roundabout between a point 1.5 times the Desirable Minimum Stopping Sight Distance from the Yield line and the Yield line itself;

c) For diverges, that length of carriageway from a point 1.5 times the Desirable Minimum Stopping Sight Distance upstream of the start of the diverge taper to the back of the diverge nose;

d) For merges, that length of carriageway from a point 1.5 times the Desirable Minimum Stopping Sight Distance upstream of the back of the merge nose to the end of the merge taper.

1.30 For the purposes of this Standard the term 'junction' shall include a lay-by (see NRA TA 69). Furthermore, Relaxations below Desirable Minimum Stopping Sight Distance are not permitted on the immediate approaches to a vehicular access other than an individual field access (see TD 41). The immediate approaches to a vehicular access are as defined for a junction in Paragraph 1.29.

Departures

1.31 In situations of exceptional difficulty which cannot be overcome by Relaxations, it may be possible to overcome them by adoption of Departures, the third tier of the hierarchy. Proposals to adopt Departures from Standard must be submitted to the National Roads Authority for approval **before** incorporation into a design layout to ensure that safety is not significantly reduced.

1.31A Although this is an Interim Advice Note and will not be adopted as an NRA DMRB Standard until monitoring of the 2+1 pilot projects has been completed, Paragraphs 1.15 to 1.31 in relation to Relaxations and Departures shall apply. Departures from Standard shall be applied for in accordance with Appendix A to Volume 6 Section 0 Part A Introduction of the NRA DMRB.

2. SIGHT DISTANCE

Stopping Sight Distance

2.1 Table 3 shows the Stopping Sight Distance (SSD) appropriate for each Design Speed.

2.2 Stopping Sight Distance shall be measured from a driver's eye height of between 1.05m and 2.00m, to an object height of between 0.26m and 2.00m both above the road surface, as shown in Figure 3. It shall be checked in both the horizontal and vertical planes, between any two points in the centre of the lane on the inside of the curve (for each carriageway on 2+1 roads and dual carriageways).

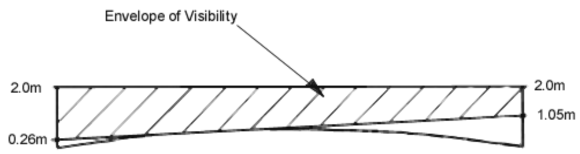


Figure 3: Measurement of Stopping Sight Distance

Full Overtaking Sight Distance

2.3 Table 3 shows for each Design Speed the Full Overtaking Sight Distance (FOSD) required for overtaking vehicles using the opposing traffic lane on single carriageway roads. Sufficient visibility for overtaking shall be provided on as much of the road as possible, especially where daily traffic flows are expected to approach the maximum design flows. FOSD is not required on motorways, dual carriageways or 2+1 roads.

2.4 FOSD shall be available between points 1.05m and 2.00m above the centre of the carriageway as shown in Figure 4, and shall be checked in both the horizontal and vertical planes.

2.5 FOSD is considerably greater than Stopping Sight Distance, and can normally only be provided economically in relatively flat terrain where the combination of vertical and horizontal alignments permits the design of a flat and relatively straight road alignment.

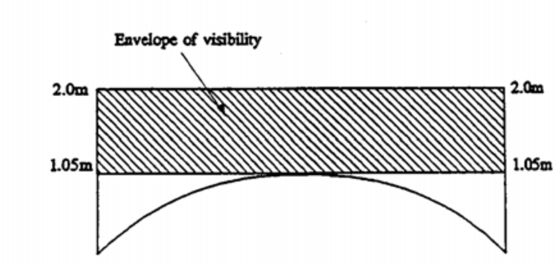


Figure 4: Measurement of FOSD

Coordinated Design of Single Carriageways

2.6 It will frequently be more economic to design a single carriageway road so as to provide clearly identifiable Overtaking Sections with FOSD in relatively level areas and with climbing lanes at hills, interspersed with Non-overtaking Sections where constraints on the alignment would result in high cost or environmental implications. The detailed standards and design considerations regarding the coordinated design of such links are given in Chapters 6 and 7. Designs which provide the driver with obvious lengths for overtaking have been found to reduce the frequency of serious accidents occurring on roads with continuous large radius curves. There is always an inherent economic trade-off between the construction and environmental costs of alternative alignments and their user benefits.

Obstructions to Sight Distance

2.7 Care shall be taken to ensure that no substantial fixed obstructions interrupt the sightlines, including road furniture such as traffic signs. However, isolated slim objects such as lamp columns, sign supports, or slim footbridge supports of width 550mm or under can be ignored. Lay-bys should, wherever possible, be sited on straights or on the outside of curves, where stopped vehicles will not obstruct sightlines.

2.7A Long bridge parapets or safety barriers on horizontal curves may obscure Stopping Sight Distance to the 0.26m object height, although the appropriate sight distance to the tops of other vehicles, represented by an object 1.05m high, will be obtained above the parapet or safety barrier. Relaxations below the Desirable Minimum Stopping Sight Distance to the low object may be appropriate in such situations.

Relaxations

2.8 In the circumstances described in Paragraphs 1.15 to 1.29, Relaxations below the Desirable Minimum Stopping Sight Distance values may be made at the discretion of the designer. The numbers of Design Speed steps permitted below the Desirable Minimum are normally as follows:

Motorways and high quality dual carriageways:		
	band A	1 step
	band B	2 steps
Other all-purpose roads:		
	bands A and B	2 steps

However, in the circumstances listed in Paragraphs 2.9 to 2.12, the scope for Relaxations shall be extended or reduced as described, provided that the resultant Relaxations do not exceed 2 Design Speed steps.

2.9 For band A roads where the Stopping Sight Distance is reduced by bridge piers, bridge abutments, lighting columns, supports for gantries and traffic signs in the verge or central reserve which form momentary obstructions, the scope for Relaxations may be extended by 1 Design Speed step.

2.10 For band A roads the scope for Relaxation of Stopping Sight Distance to the 0.26m object height, for sight lines passing in front of long obstructions such as bridge parapets or safety barriers, may be extended by 1 Design Speed step, provided the appropriate Stopping Sight Distance (one Design Speed step below SSD or longer) is available to the high object (see also Paragraph 2.7A).

2.11 On or near the bottom of long grades on dual carriageways and 2+1 roads steeper than 3% and longer than 1.5km, the scope for Relaxations shall be reduced by 1 Design Speed step. Conversely, at or near the top of up gradients on single carriageways steeper than 4% and longer than 1.5 km, the scope for Relaxation may be extended by 1 step due to reduced speeds uphill.

2.12 The scope for Relaxations shall be reduced by 1 Design Speed step immediately following an Overtaking Section on single carriageway roads (see Paragraphs 7.5 to 7.16).

2.13 Relaxations below Desirable Minimum Stopping Sight Distance, other than Relaxations to the low object at central reserve safety barriers (see Paragraph 2.7A), are not permitted on the immediate approaches to junctions as defined in Paragraph 1.29.

3. HORIZONTAL ALIGNMENT

Road Camber

3.1 On sections of road with radii greater than that shown in Table 3 for Minimum R without elimination of adverse camber & transitions (i.e. $V^2/R < 5$), the crossfall or camber should be 2.5%, normally from the centre of single carriageways, within the central reserve on 2+1 roads, or from the central reserve of dual carriageways to the outer channels. At junctions other than roundabouts, the cross-section of the major road shall be retained across the junction, and the side road graded into the channel line of the major road. On horizontal curves, adverse camber shall be replaced by favourable crossfall of 2.5% or more when the radius is less than that shown in Table 3 for 'Minimum R without elimination of adverse camber & transitions' (i.e. $V^2/R > 5$). However, it will frequently be necessary to eliminate adverse camber on larger radii for aesthetic or drainage reasons.

3.1A On minor roads where the quality of road pavement laying is unlikely to be high, the minimum crossfall should be 3%.

Superelevation

3.2 On radii less than those shown in Table 3 for Minimum R with superelevation of 2.5% (i.e. $V^2/R > 7.07$), superelevation shall be provided, such that:

$$S = \frac{V^2}{2.828 \times R}$$

where :

V = Design Speed, km/h

R = Radius of Curve, m

S = Superelevation, %.

On Rural Roads superelevation shall not exceed 7%. On Urban Roads with at-grade junctions and accesses, superelevation shall be limited to 5%.

Figure 5 shows the appropriate superelevation for the range of Design Speeds. Sharper radii than the Desirable Minimum values shown in Table 3 result in steep crossfalls which should be avoided if possible. It is essential to maintain adequate skidding resistance and good drainage at all superelevations.

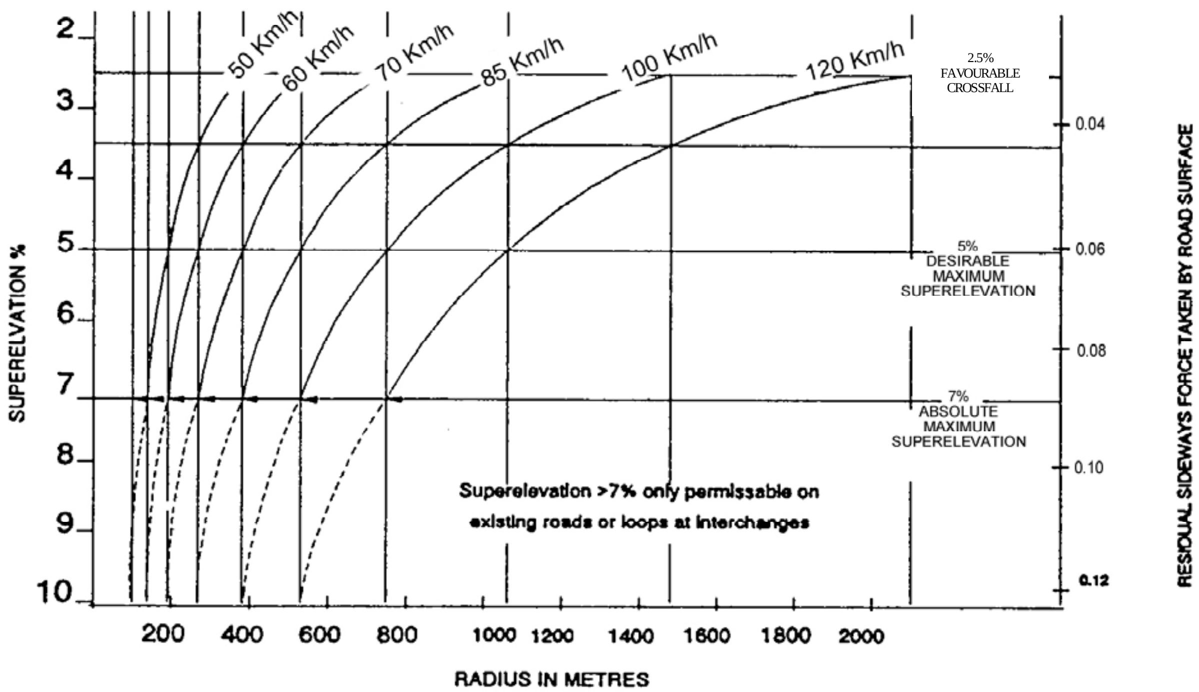


Figure 5: Superelevation of Curves

Desirable Minimum Radius

3.3 The Desirable Minimum radii, corresponding to a superelevation of 5% (i.e. $V^2/R = 14.14$) are shown in Table 3.

Relaxations

3.4 In the circumstances described in Paragraphs 1.16 to 1.28, Relaxations of up to 2 Design Speed steps below the Desirable Minimum values may be made at the discretion of the designer for all road types. However, for roads in Design Speed band B in the circumstances listed in Paragraphs 3.5 and 3.6, the scope for Relaxations shall be extended or reduced as described, provided that the resultant Relaxations do not exceed 2 Design Speed steps.

3.5 On or near the bottom of long grades on dual carriageways steeper than 3% and longer than 1.5km the scope for Relaxations shall be reduced by 1 Design Speed step. Conversely, at or near the top of up gradients on single carriageways steeper than 4% and longer than 1.5 km, the scope for Relaxations may be extended by 1 step due to reduced speeds uphill.

3.6 The scope for Relaxations shall be reduced by 1 Design Speed step immediately following an Overtaking Section on single carriageway roads (see Paragraphs 7.5 to 7.16).

Appearance and Drainage

3.7 Superelevation shall not be introduced, nor adverse camber removed, so gradually as to create large almost flat areas of carriageway, nor so sharply as to cause discomfort or to kink the edges of the carriageway. A satisfactory appearance can usually be achieved by ensuring that the carriageway edge profile does not vary in grade by more than about 1% from that of the line about which the carriageway is pivoted, and by ample smoothing of all changes in edge profile. In general on motorways and high quality dual carriageways, a smoother edge profile should be provided by reducing the variation in grade of the edge profile to a maximum of 0.5% where feasible, i.e. where local drainage conditions permit. Care should be taken to ensure that a minimum longitudinal gradient of at least 0.5% is maintained wherever superelevation is to be applied or reversed. However, in some difficult areas even the above requirements can lead to drainage problems, e.g. where the superelevation is applied against the longitudinal gradient. It may be necessary to modify the horizontal alignment thereby moving the superelevation area, to increase the variation in grade of the edge profile, or to apply a rolling crown. Areas susceptible to such drainage problems should be identified at an early stage in the design process, before the horizontal alignment is fixed.

Application of Superelevation

3.8 Progressive superelevation or removal of adverse camber shall generally be achieved over or within the length of the transition curve from the arc end (see also Paragraph 3.17). On existing roads without transitions, between ½ and $^2/_3$ of the cant shall be introduced on the approach straight and the remainder at the beginning of the curve.

Widening on Curves

3.9 Pavement widening at curves on links and on the main line through junctions is required on low radius curves to allow for the swept path of long vehicles.

3.10 For carriageways of standard width (with lane widths of 3.5m, 3.65m or 3.75m depending on the road type), each lane shall be widened to 3.95m when the radius is between 90m and 150m.

3.10A For carriageways of standard width, an increased width of 0.15m per lane shall be applied when the radius is between 150m and 1,000m. However, at these radii lane widths do not need to be widened beyond 3.65m.

3.11 For carriageways less than the standard widths, widening shall be:

a) 0.6m per lane where the radius is between 90m and 150m, subject to maximum carriageway widths of 3.95m, 7.9m, 11.9m and 15.8m (for 1, 2, 3 and 4 lanes respectively).

b) 0.5m per lane where the radius is between 150m and 300m, subject to maximum carriageway widths of 3.95m, 7.9m, 11.9m and 15.8m (for 1, 2, 3 and 4 lanes respectively).

c) 0.3m per lane, where the radius is between 300m and 400m, subject to maximum carriageway widths of 3.95m, 7.9m, 11.9m and 15.8m (for 1, 2, 3 and 4 lanes respectively).

3.12 Radii less than 90m on the mainline are Departures from Standard. For these and all other junction elements, widening should be in accordance with TD 42.

3.13 The extra width should be applied uniformly along the transition curve. In the improvement of existing curves the widening should generally be made on the inside of curves.

Lane Width Reductions at Pinch Points

3.14 At points of particular difficulty on Wide Dual Carriageways, where full lane widths cannot be achieved, a reduction from 3.75m to 3.50m is permitted as a Relaxation provided that the radius of curvature exceeds 1,000m. Points where such a Relaxation is likely to be most applicable are around the urban fringe, at sites with difficult topography or in historic or conservation areas. This Relaxation shall not apply on new single carriageway roads.

Transitions

3.15 Transition curves shall be provided on any curve the radius of which is less than that shown in Table 3 for Minimum R without elimination of adverse camber and transitions (i.e. $V^2/R < 5$).

3.16 Length of Curve: The basic transition length shall be derived from the formula:

$$L = \frac{V^3}{46.7 \times q \times R}$$

where:

L = Length of transition (m)

V = Design Speed (km/h)

q = Rate of increase of centripetal acceleration (m/sec^3) travelling along curve at constant speed V

R = Radius of curve (m).

q should normally not exceed 0.3m/sec^3. However, in difficult cases the value of q may be increased up to 0.6 m/sec^3 as a Relaxation. On curves which are sub-standard for the appropriate Design Speed, the length of transition should normally be limited to $\sqrt{(24R)}$ metres.

3.17 Application of Superelevation: Superelevation or elimination of adverse camber shall generally be applied on or within the length of the transition curve from the arc end. The basic transition appropriate to the Design Speed, however, will often result in insufficient transition length to accommodate superelevation turnover: in such cases longer transitions should be provided to match the superelevation design.

The Effect of Sight Distance at Horizontal Curves

3.18 Stopping Sight Distance: When the road is in a cutting, or at bridge crossings, it may be necessary to widen verges or increase bridge clearances to ensure that the appropriate Stopping Sight Distance is not obstructed. Figure 6 shows the maximum central offset required with varying horizontal curvature, in order to maintain the Design Speed related Stopping Sight Distances. It can be seen that extensive widening of verges and structures, or central reserves with hedges or safety barriers, would be required to maintain Desirable Stopping Sight Distances on horizontal radii below Desirable Minimum. Where a road is on embankment, however, visibility will be available across the embankment slope. However, it must be ensured that the sight distance is not obscured by landscape planting.

3.19 Full Overtaking Sight Distance: Figure 7 shows the maximum central offset required with varying horizontal curvature, in order to maintain the Design Speed related FOSDs. It can be seen that the higher requirements of FOSD result in extensive widening of verges for all but relatively straight sections of road.

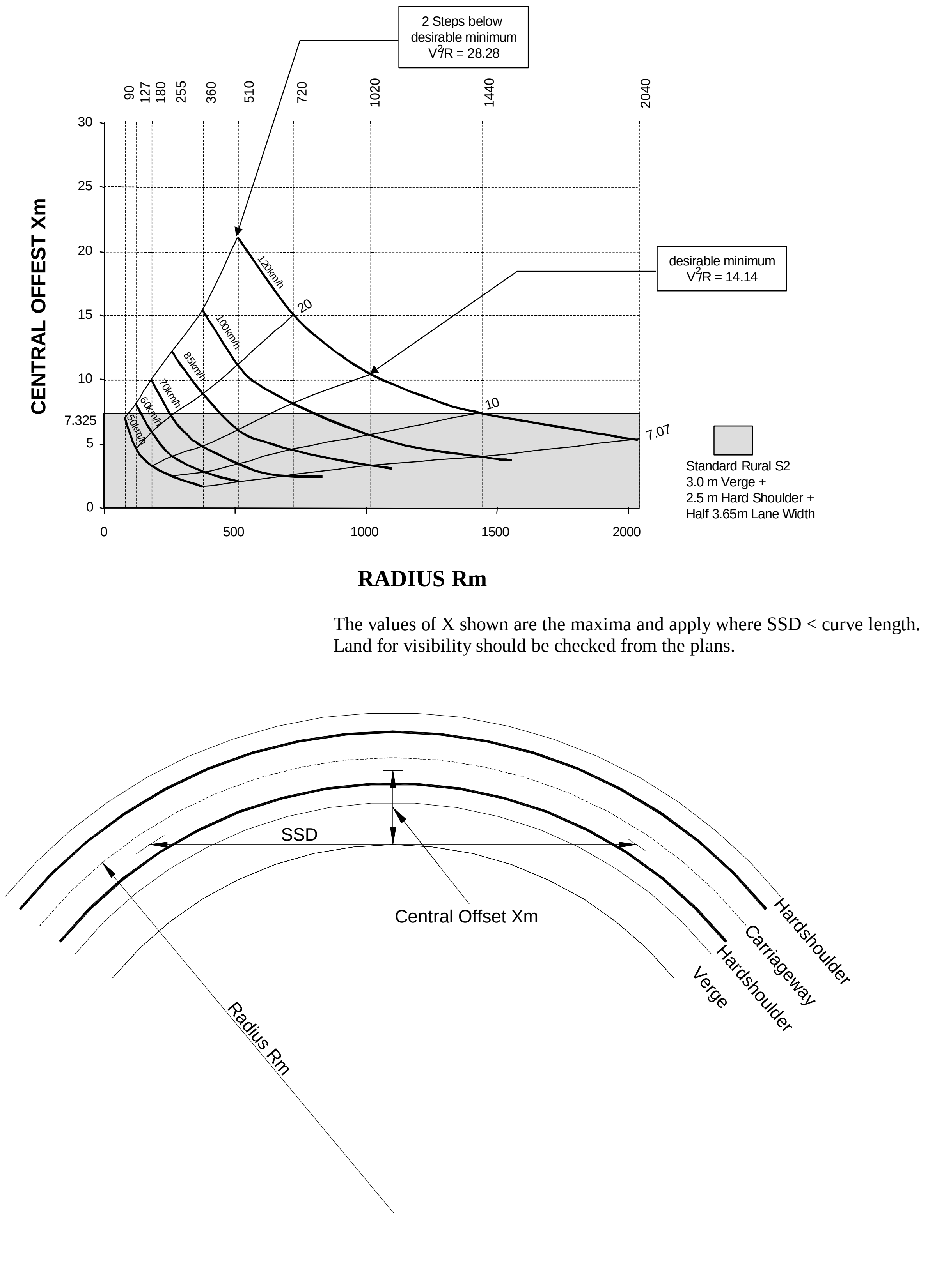
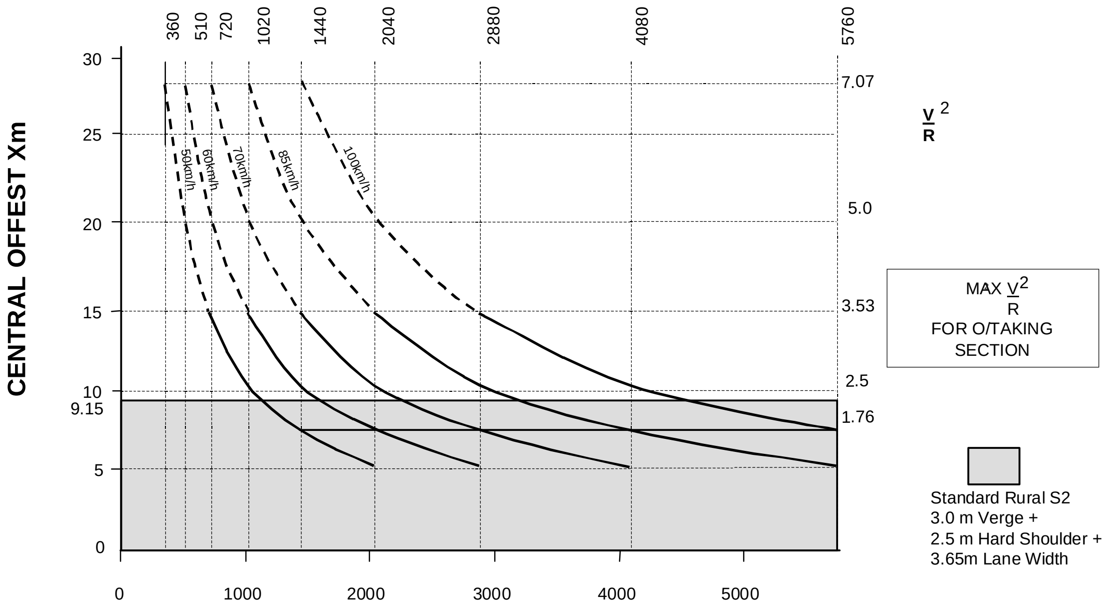


Figure 6: Verge Widening for Desirable Minimum Stopping Sight Distance



RADIUS Rm

The values of X are the maxima and apply where FOSD < curve length.
Land for visibility should be checked from the plans.

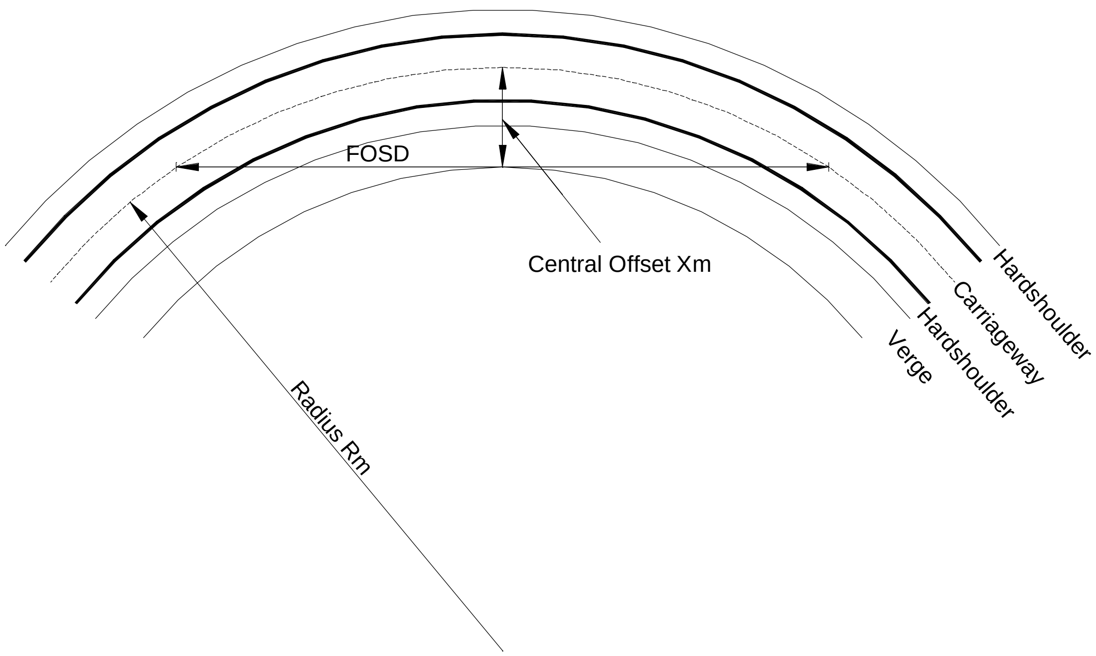


Figure 7: Verge Widening for Full Overtaking Sight Distance

4. VERTICAL ALIGNMENT

Gradients

4.1 Maximum Gradients: The Desirable Maximum gradient for design shall be:

	Desirable Max Grade
Motorways and High Quality Dual Carriageways	3%
Other AP Dual Carriageways	4%
2+1 Roads (on new alignments)	4%
2+1 Roads (on-line upgrading of existing roads)	5%
Single Carriageways:	
National and Regional Roads	5%
County Roads	6%

However, in hilly terrain steeper gradients will frequently be required, particularly where traffic volumes are at the lower end of the range.

4.2 Effects of Steep Gradients: In hilly terrain the adoption of gradients steeper than Desirable Maximum could make significant savings in construction or environmental costs, but would also result in higher user costs, i.e. by delays, fuel and accidents. Slightly steeper gradients are, therefore, permitted as Relaxations. There is, however, a progressive decrease in safety with increasingly steeper gradients. Departures from Standards will, therefore, be required for any proposals to adopt gradients steeper than the following:

	Max Grade with Relaxation
Motorways and High Quality Dual Carriageways	4%
Other AP Dual Carriageways	5%
2+1 Roads (on new alignments)	5%
2+1 Roads (on-line upgrading of existing roads)	6%
Single Carriageways:	
National and Regional Roads	6%
County Roads	8%

4.3 Minimum Gradients: For effective drainage with kerbed roads a minimum gradient of 0.5% should be maintained wherever possible. In flatter areas, however, the vertical alignment should not be manipulated by the introduction of vertical curvature simply to achieve adequate surface water drainage gradients. Drainage paths must be provided by false channel profiles with minimum gradients of 0.5%. False channels may be avoided by using over-edge drainage (to filter drains or surface channels or ditches) where kerbs are inappropriate, e.g. in rural areas.

Vertical Curves

4.4 General: Vertical curves shall be provided at all changes in gradient. The curvature shall be large enough to provide for comfort and, where appropriate, sight distances for safe stopping at Design Speed. The use of the permitted vertical curve parameters will normally meet the requirements of visibility. However Stopping Sight Distance should always be checked because the horizontal alignment of the road, presence of crossfall, superelevation or verge treatment and features such as signs and structures adjacent to the carriageway will affect the interaction between vertical curvature and visibility.

4.5 K Values: Curvature shall be derived from the appropriate K value in Table 3. The minimum curve lengths can be determined by multiplying the K values shown by the algebraic change of gradient expressed as a percentage, e.g. +3% grade to -2% grade indicates a grade change of 5%. Thus for a Design Speed of 120 km/h, the length of a crest curve would be:-

Desirable Min = 5 x 182 = 910m

One step below Des Min = 5 x 100 = 500m.

4.6 Crest Curves: There are two factors that affect the choice of crest curvature: visibility and comfort. At all Design Speeds in Table 3 the Desirable Minimum crest in the road will restrict forward visibility to the Desirable Minimum Stopping Sight Distance before minimum comfort criteria are approached, and consequently the Desirable Minimum crest curves are based upon visibility criteria.

4.6A The use of crest curves with K values greater than Desirable Minimum but less than FOSD Overtaking Crest on single carriageway roads, in combination with a straight or nearly straight horizontal alignment (such that the section of road could form part of a Two-lane Overtaking Section in the horizontal sense), is a Departure from Standards (see Paragraph 7.19).

4.7 Sag Curves: Daytime visibility at sag curves is usually not obstructed unless overbridges, signs or other features are present; this also applies to night-time visibility on roads that are lit. However, sag curvature does affect night-time visibility on unlit roads. The Desirable Minimum sag curves are based on a conservative comfort criterion (0.21 m/sec^2 maximum vertical acceleration); the resultant sag curves approximate to those using a headlamp visibility criterion assuming a 1.5^o upward spread of the light beam. The sag curves for 1 Design Speed step below Desirable Minimum are based on the conventional comfort criterion of 0.3 m/sec^2 maximum vertical acceleration. The adoption of this approach results in the sag curve K values being less than or equal to the equivalent crest curve K values at all the Design Speeds in Table 3.

4.8 Grass Verges: Where, at crests, the sight line crosses the verge, consideration shall be given to the design of a lower verge profile in order to allow for an overall height of grass of 0.5m.

Relaxations

4.9 Crest Curves: In the circumstances described in Paragraphs 1.16 to 1.28, Relaxations below the Desirable Minimum values may be made at the discretion of the designer. The number of Design Speed steps permitted below the Desirable Minimum are normally as follows:

Motorways and high quality dual carriageways:

	band A	1 step
	band B	2 steps

Other all-purpose roads:

	bands A and B	2 steps.

However, in the circumstances listed in Paragraphs 4.10 to 4.12 the scope for Relaxations shall be extended or reduced as described, provided that the resultant Relaxations do not exceed 2 Design Speed steps.

4.10 At or near the top of up gradients on single carriageways steeper than 4% and longer than 1.5 km, the scope for Relaxations may be extended by 1 Design Speed step due to reduced speeds uphill.

4.11 The scope for Relaxations shall be reduced by 1 Design Speed step immediately following an Overtaking Section on single carriageway roads (see Paragraphs 7.5 to 7.16).

4.12 For band A roads when the crest curve is within a straight section the scope for Relaxations may be extended by 1 Design Speed step.

4.13 Relaxations below Desirable Minimum are not permitted on the immediate approaches to junctions as defined in Paragraph 1.29.

4.14 Sag Curves: In the circumstances described in Paragraphs 1.16 to 1.28, Relaxations below the Desirable Minimum values may be made at the discretion of the designer. The number of Design Speed steps permitted below the Desirable Minimum are normally as follows:

Motorways and high quality dual carriageways:

	band A	1 step
	band B	2 steps

Other all-purpose roads:

	bands A and B	2 steps.

However, in the circumstances listed in Paragraph 4.16, the scope for Relaxations shall be reduced as described.

4.15 (Not used.)

4.16 The scope for Relaxations shall be reduced by 1 Design Speed step immediately following an Overtaking Section on single carriageway roads (see Paragraphs 7.5 to 7.16).

4.17 Relaxations more than one Design Speed step below Desirable Minimum are not permitted on the immediate approaches to junctions as defined in Paragraph 1.29.

5. CLIMBING LANES

Introduction

5.1 A climbing lane is an additional lane added to a road in order to improve capacity and/or safety because of the presence of a steep gradient. The steep gradient is the primary reason for adding the lane. On single carriageways and 2+1 roads climbing lanes provide two lanes for uphill traffic whilst the opposing traffic is partially or fully confined to one lane: they, therefore, provide overtaking opportunities. On dual carriageways the need for climbing lanes is less, since overtaking opportunities are greater, but they can alleviate congestion at higher traffic flows.

5.2 This chapter outlines the design principles and other factors which should be considered by designers for the introduction of climbing lanes into new or existing carriageways.

5.3 On single carriageway roads, a climbing lane should be considered if it can be justified (see Paragraphs 5.11 and 5.12) on hills with gradients greater than 2% and longer than 500m. Justification is unlikely to be achieved where the traffic flows are less than 4,000 Annual Average Daily Traffic (AADT) in the design year.

5.4 On dual carriageway roads (including motorways), a climbing lane should be considered if it can be justified (see Paragraphs 5.11 and 5.12) on hills with gradients greater than 3% and longer than 500m. Justification is unlikely to be achieved where the traffic flows in the design year are less than 75% of the capacity given in Table 4 for the relevant category of road.

5.4A On 2+1 roads, a climbing lane should be considered if it can be justified (see Paragraph 5.49) on hills with gradients greater than 2% and longer than 500m.

5.5 In some cases a detailed scheme appraisal, as outlined in Paragraphs 5.6 to 5.17, may provide justification for a climbing lane even when the above criteria are not met.

Scheme Appraisal

5.6 Consideration of the need for and justification of a climbing lane should form an integral part of the development of a scheme. Assessment, consultation and design should be an iterative process, considering the appropriateness and significance of impacts measured against the scheme objectives. Appraisal of the effects of a climbing lane should consider:

- Economy: reduction in travel times, vehicle operating costs and journey time reliability;

- Environment: effects on environmental intrusion, reduction in driver frustration, noise and air pollution;

- Safety: reduction in accidents.

5.7 Climbing lanes add another optional element to the treatment of vertical alignment. They may allow steeper, shorter gradients to be considered, which would reduce earthworks, be less intrusive to the local environment, and offset the cost of the wider road. However, from a traffic benefit viewpoint, the option of flattening gradients may often be preferable. The implications of long steep gradients on the downhill carriageway should also be considered.

5.8 Assessment of Impacts: The provision of an additional uphill lane should provide benefits to travellers by diminishing delays caused by slow-moving traffic. The effect of adding a lane is two-fold: some traffic is able to move over to a faster lane, thereby gaining a significant speed advantage, and the consequent reduction in traffic in the left-hand lane can enable speeds to increase in this lane. Where traffic flows are approaching capacity, gradients without climbing lanes can be pinch points where congestion starts. Where flows are less, the economic benefits are likely to be less substantial but the climbing lane can also be viewed as a safety measure, creating a safer overtaking opportunity and reducing driver frustration.

5.9 Where a climbing lane is to be added to an existing carriageway, data should be collected and "Before" surveys carried out if appropriate.

5.10 On a new road, the introduction of a steep gradient with a climbing lane should be compared with an alternative with lesser gradients and no climbing lane. The latter may have greater costs and impacts due to the need for more extensive earthworks.

5.11 Economy: The criteria for provision of climbing lanes (see Paragraphs 5.18 and 5.19 for single carriageway roads, Paragraph 5.39 for dual carriageway roads and Paragraph 5.49 for 2+1 roads) will ensure that the climbing lane is economically justified in most cases, provided there are no high cost elements along the relevant length of road.

5.12 Where there are high cost elements or other factors which make economic appraisal appropriate, an economic appraisal should be undertaken, considering a Do Something (climbing lane) option against the Do Nothing (no climbing lane), as well as an assessment of alternative climbing lane lengths and slope configurations. The method of economic appraisal to be adopted should be agreed with the National Roads Authority.

5.13 Environment: Climbing lanes can have an impact on the environment in a number of ways and environmental issues need to be considered as an integral part of the design and appraisal process. The likely impact on, for example, wildlife will be neutral or negative if additional land-take is necessary. However, the impact may be positive if an increased gradient with diminished earthworks leads to less land-take and reduced visual intrusion.

5.14 Driver frustration should form part of the environmental appraisal process for single carriageway roads. Whilst useful engineering data relating to driver frustration are scarce, careful consideration should be given to the provision of adequate overtaking opportunities (see Paragraphs 7.5 to 7.24).

5.15 Safety: Climbing lanes help to relieve driver frustration and provide a safer overtaking environment, particularly on single carriageway roads. As a guide, the presence of a climbing lane on a single carriageway road can be expected to reduce the accident rate by 25%.

5.16 Factors which tend to make the road less safe and which, therefore, should be avoided include: sharp bends, poorly marked and located junctions, short climbing lane sections, and short or unusual entry or exit tapers. In particular, the exit taper should not be located in the vicinity of junctions or sharp bends.

5.17 Where the criteria of Paragraphs 5.11 and 5.12 are not met, an assessment should be made, taking all factors into account, including the effects on the road user. Whilst the quantifiable economic benefits of the climbing lane may not be quite sufficient to justify its provision, the resulting loss of Net Present Value may be only minor, and thus a small price to pay for the unquantifiable benefits the climbing lane would provide to traffic, such as relieving the frustration of platoons caused by slow moving heavy goods vehicles (see Paragraph 7.24). An example of a situation where such a situation may occur is a hill slightly shorter than 500m where a climbing lane would provide a useful overtaking opportunity.

Single Carriageways

5.18 Criteria for Provision: On single carriageway roads on hills with gradients (G = 100H/L) greater than 2% and longer (L) than 500m the following criteria may be used to determine the justification for a climbing lane as an alternative to economic appraisal:

a) On single carriageways without hard shoulders (or with narrow hard shoulders), Figure 5/1 may be used. The solid curves in Figure 5/1 show the height risen, H, of a hill required to justify the provision of a climbing lane, according to the design year traffic forecast. The figure assumes the standard cost of a climbing lane in relatively easy terrain.

b) On single carriageways with full width hard shoulders (2.5m or more), the climbing lane should replace the hard shoulder, with little or no additional width (see Paragraph 5.22). As the cost of provision of the climbing lane in such cases will be small, climbing lanes should generally be provided on gradients greater than 2% wherever the risen height (H) exceeds 15m and the traffic flow will exceed 6,000 AADT in the design year. This is shown by the dashed line in Figure 5/1.

In both cases, the height risen (H) and length (L) shall be calculated between two standard points on the hill as illustrated in Figure 5/2.

5.19 On single carriageways without hard shoulders, where there are high cost elements involved such as heavy earthworks, bridgeworks or environmental effects (which would invalidate the average cost assumptions of Figure 5/1), it may be uneconomic or undesirable to make full provision. It may be preferable to adopt a Departure from Standards, by providing the climbing lane partially within the normal verge width/marginal strip to reduce the high cost implications, rather than omit the climbing lane altogether.

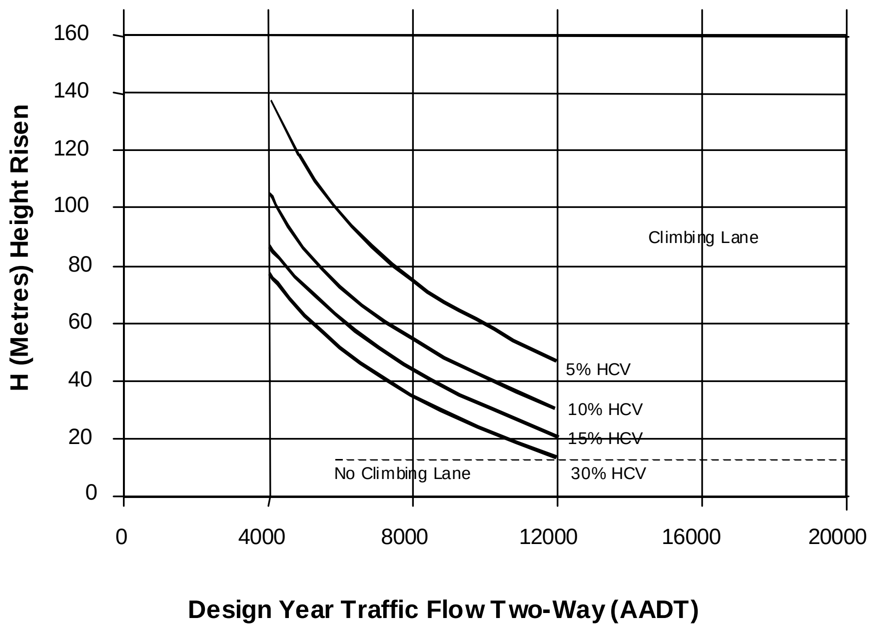


Figure 5/1: Single Carriageway Climbing Lanes

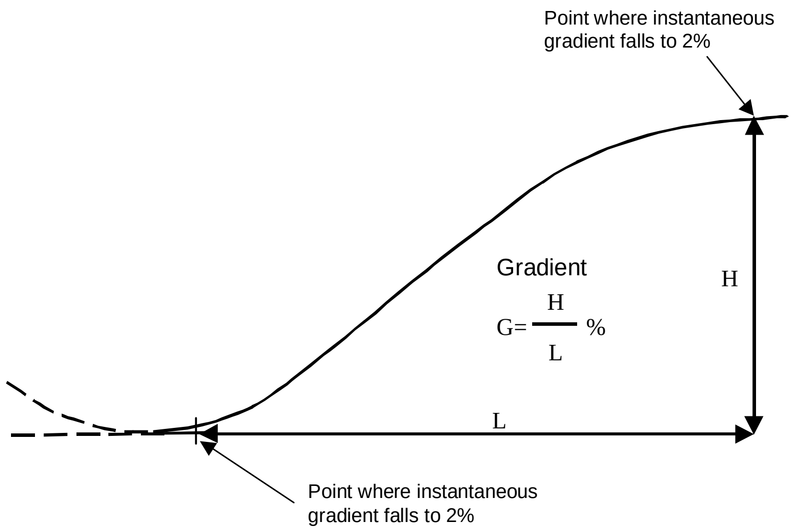
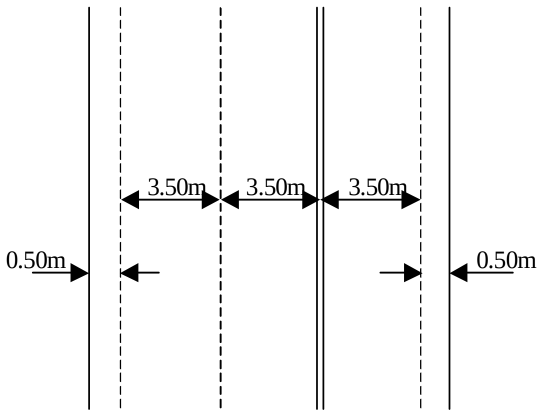


Figure 5/2: Definitions for Climbing Lanes

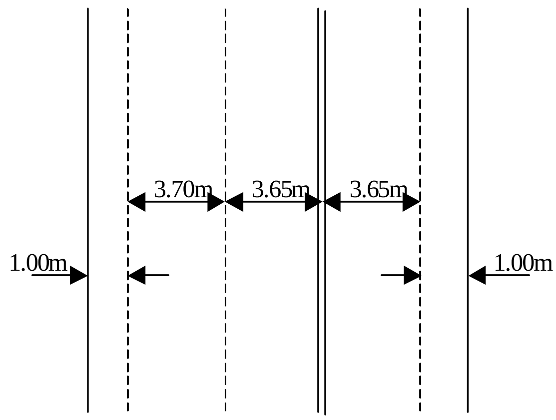
5.20 Length of Climbing Lanes: A climbing lane should not be provided unless the length of full width climbing lane section is a minimum of 600m. This length will normally be provided where the length 'L', the distance with gradients in excess of 2%, is 500m or more. Where a climbing lane is being provided on a shorter hill, for example to provide an overtaking section, it should be extended to a minimum of 600m. However, care should be taken with the design of the end taper, since the speed of vehicles in the climbing lane will increase as the hill flattens. Short climbing lanes have a higher accident risk that is exacerbated by bends in the road. High accident rates are associated with average bendiness (irrespective of the climbing lane length) in excess of 50degs/km.

5.21 Climbing lane road markings tend to confine downhill traffic to a single lane, unless there is ample forward visibility unobstructed by slow moving vehicles in the climbing lane. Where the length of a climbing lane exceeds about 3 km, therefore, it is important that some sections are provided with a straight or large radius right hand curvature in order to provide an Overtaking Section for downhill traffic (see Paragraph 7.13).

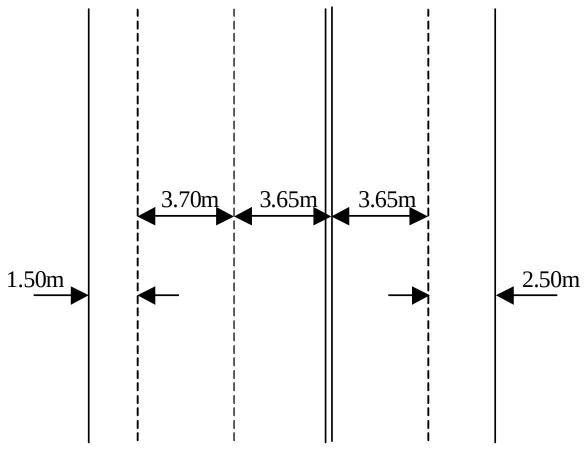
5.22 Lane Widths: The cross-sections of single carriageways including climbing lanes shall be as shown in Figures 5/3(a), (b) and (c).



(a): On Reduced Single Carriageway



(b): On Standard Single Carriageway



(c): On Wide Single Carriageway

Notes:

1. For standard road cross-sections, see NRA TD 27.

2. The overall width of paved surface in case (c) should be equal to that without a climbing lane but including hard shoulders.

Figure 5/3: Climbing Lanes on Single Carriageways

5.23 Layout at Start of Climbing Lane: The full width of the climbing lane shall be provided at a point 'S', 100m uphill from the 2% point of sag curve, and preceded by a taper of 1/50, as shown in Figure 5/4. The length of the taper shall be such that traffic in the lane which is required to experience the greatest lateral shift over the length of the taper does so at 1/50. The alignment at the commencement of the climbing lane shall encourage drivers to follow the nearside channel unless overtaking. The taper shall therefore provide a smooth transition, by utilising the road curvature to develop the extra width, wherever possible. Where the curvature is used in this way, the length of taper may be reduced to 1/40. Specific signing of the climbing lane will not be necessary.

5.24 Climbing lanes may also be inserted directly into the exit lane of a roundabout where appropriate.

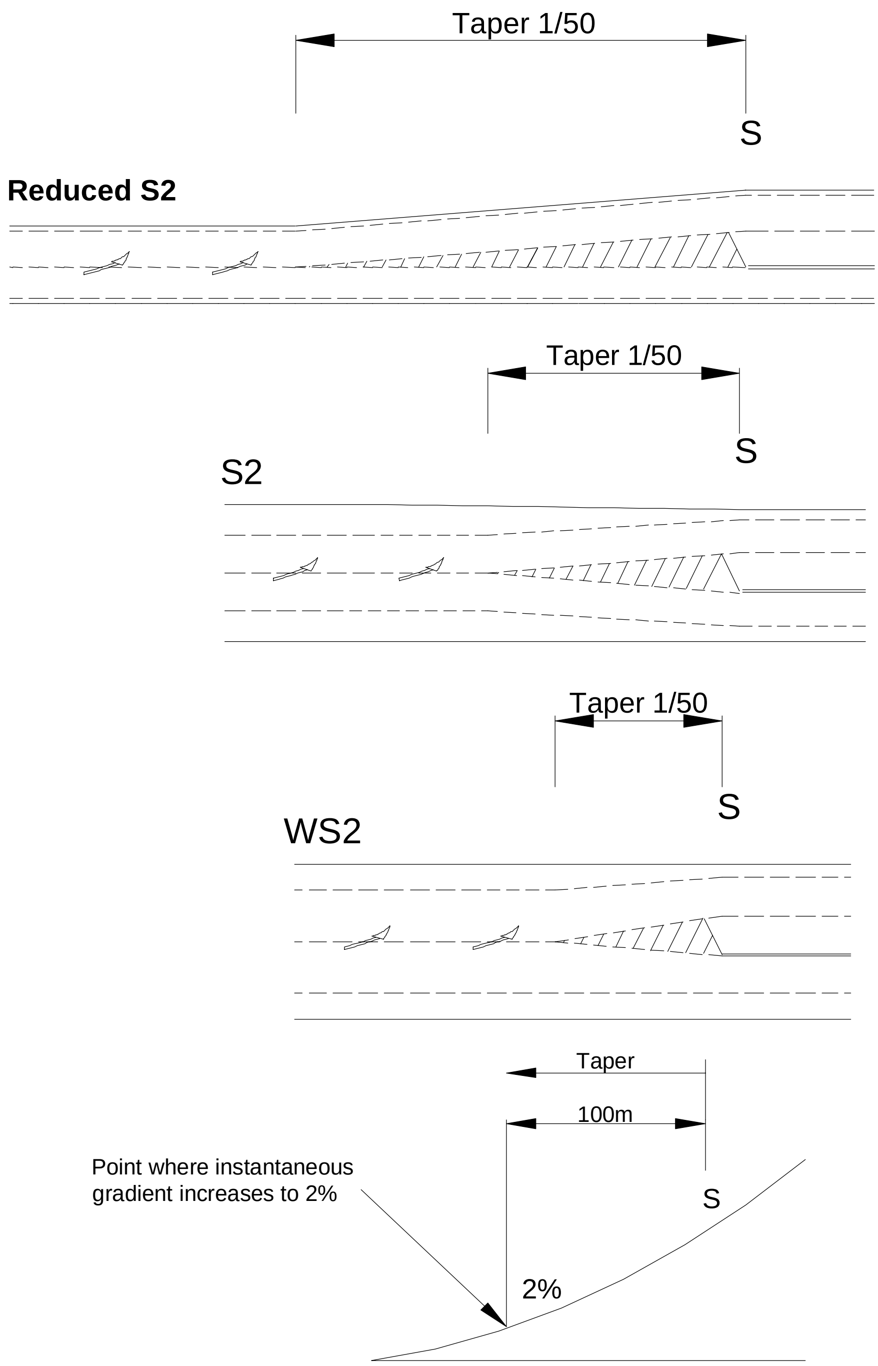


Figure 5/4: Start of Climbing Lane

5.25 Layout at End of Climbing Lane: The full width of the climbing lane shall be maintained until a point 'F', at least 220m beyond the point at which the gradient reduces to 2% at the crest curve. After point F the carriageway width shall be tapered back to the normal two-lane width at a taper of 1:70 for the lane marking which has the greatest lateral shift, as shown in Figure 5/5. On a reduced single carriageway, the full width of the paved surface (including hard strips) shall be maintained up to the end of the taper. A 200m length of hard shoulder shall then be provided on the climbing lane side of a reduced S2, followed by a taper of 1:70 to the normal paved width (see Figure 5.5).

5.26 The alignment at the end of the climbing lane shall place the onus on the driver in the right –hand lane to rejoin the continuing lane. The taper shall provide a smooth transition in the same manner as that at the start of the climbing lane. Where the road curvature is used to provide a smooth transition, the lengths of tapers may be reduced to 1:40 as a Relaxation. Advance warning signs shall be provided as shown in Figure 5/5. Care should be taken to ensure that the return to a single lane does not coincide with junctions or a sharp curve.

5.27 Consideration should be given to extending the distance between the 2% point and point F, the end of the full width climbing lane, in the following circumstances:

a) Where an extension enables traffic to merge more safely;

b) If an existing junction is in the vicinity of the end taper area;

c) If the climbing lane is part of an overall route strategy for overtaking (see Paragraphs 7.20 to 7.24) and the climbing lane is extended to maximise overtaking opportunities;

d) If a high proportion of HCVs or slow moving vehicles currently cause problems at the end taper of an existing climbing lane, the lane may be extended where heavy vehicles are picking up speed as the road begins to descend from the crest of the hill.

5.28 Where the climbing lane is extended the taper arrangement at the end of the lane shall be as shown in Figure 5/5.

5.29 The climbing lane may terminate at a roundabout where appropriate, with the overtaking lane becoming the right hand entry lane into the roundabout. If the climbing lane would terminate within 500m of the roundabout, it should be continued to the roundabout.

5.30 Junctions: Careful consideration should be given with respect to the location of junctions along the length of the climbing lane. On new roads, junctions shall not be provided within the length of the climbing lane (including the tapers): any proposal for such a junction would require a Departure from Standard. On existing roads, junctions within the length of the climbing lane should be closed where possible and accesses and side roads should be re-routed.

5.31 Signing: Clear signing and road markings at the end of a climbing lane are very important, to ensure that drivers are aware of the potential 'change of lane' manoeuvres that will be taking place ahead. This is important for both safety and the efficient operation of the climbing lane.

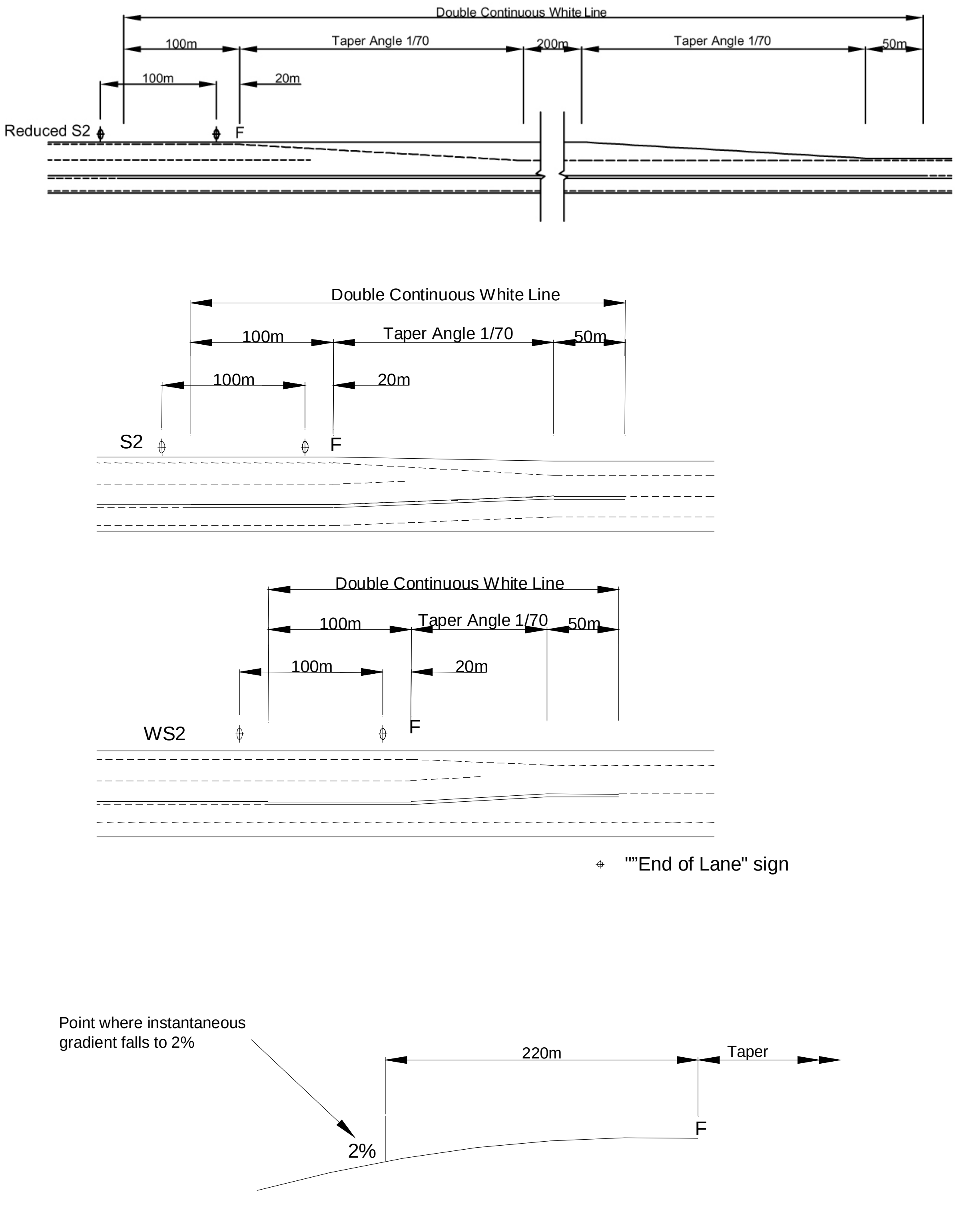
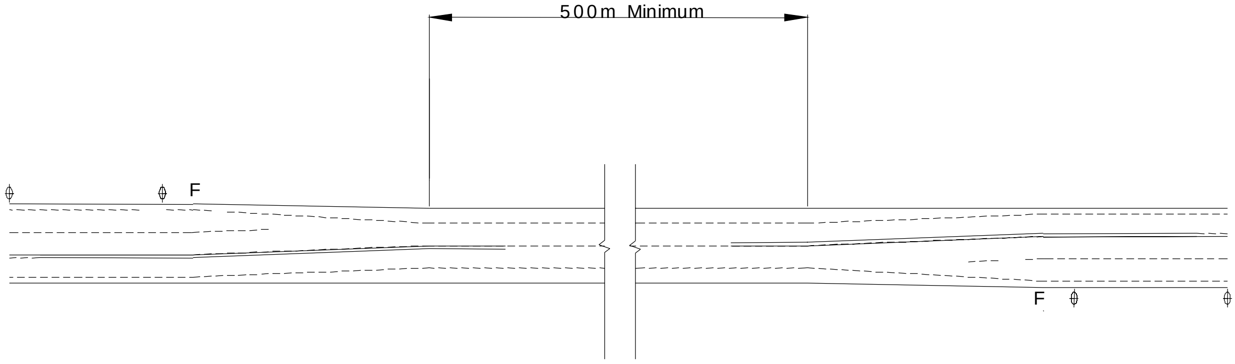
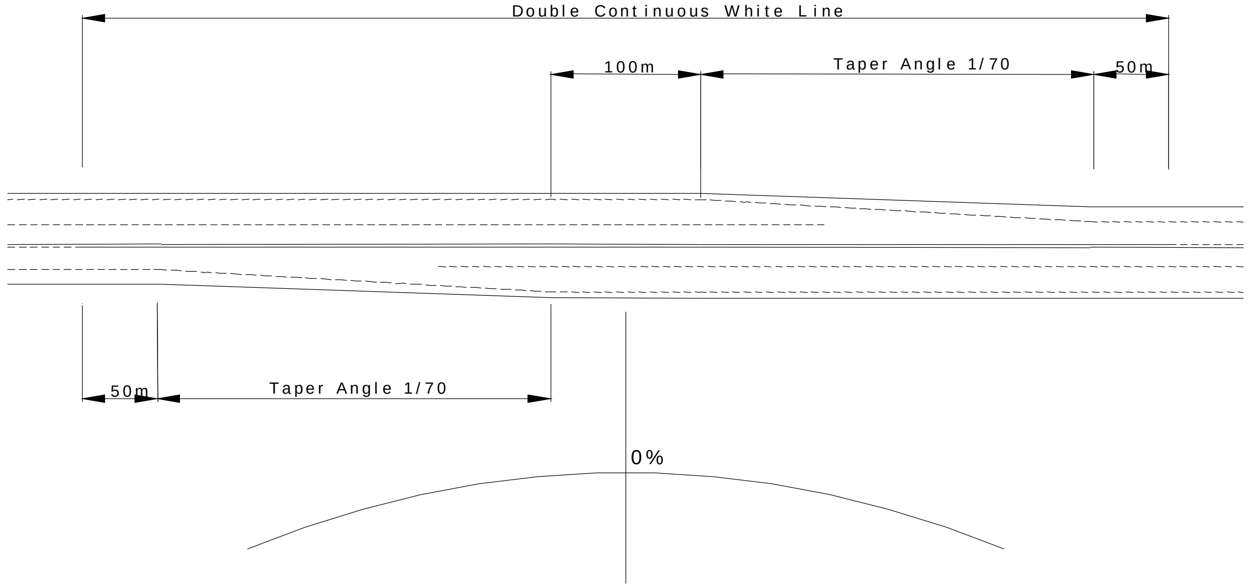


Figure 5/5: End of Climbing Lane

5.32 Layout at crests: Where there are climbing lanes on both sides of the hill, and profile conditions would lead to a conventional road layout between ends of tapers of less than 500m in length (see Figure 5.6a), the climbing lanes shall be extended to provide a length of four lane road at the summit: the detailed layout of a four lane crest is shown in Figure 5/6b. The overlap of the full width climbing lanes should not be less than 100m. The treatment of lanes, hard shoulders and hard strips should follow Figures 5/3 and 5/5 for the appropriate carriageway standard.



(a): Crest Curve Between Separated Climbing Lanes



(b): Crest Curve With Overlapping Climbing Lanes

Figure 5/6: Crest With Two Climbing Lanes

5.33 Layout at Sags: Where there are climbing lanes either side of a sag curve, and profile conditions would lead to a conventional 2 lane road layout between starts of tapers of less than 500m in length, the climbing lanes shall be extended downhill until they meet, as illustrated in Figure 5/7. The treatment of lanes, hard shoulders and hard strips should follow Figure 5/3 for the appropriate carriageway standard.

5.34 Sight Distance Requirements: Climbing lanes on single carriageways do not require Full Overtaking Sight Distance, but the Desirable Minimum Stopping Sight Distance shall be provided throughout. In difficult circumstances a one step Relaxation below Desirable Minimum SSD may be provided. Care should be taken, however, in the design of the crest curve. If vehicles on the crest approaching the downhill section are provided with a high visibility crest curve, there is a possibility of subsequent abuse of the priority rule. The crest curve should be designed to a K value of (or slightly more than) one Design Speed step below Desirable Minimum. A double continuous line marking should be provided as in Figure 5/5 to establish clearly the climbing lane priority. If sight distance increases beyond the crest, the marking should then become continuous/broken to permit some overtaking in the downhill direction.

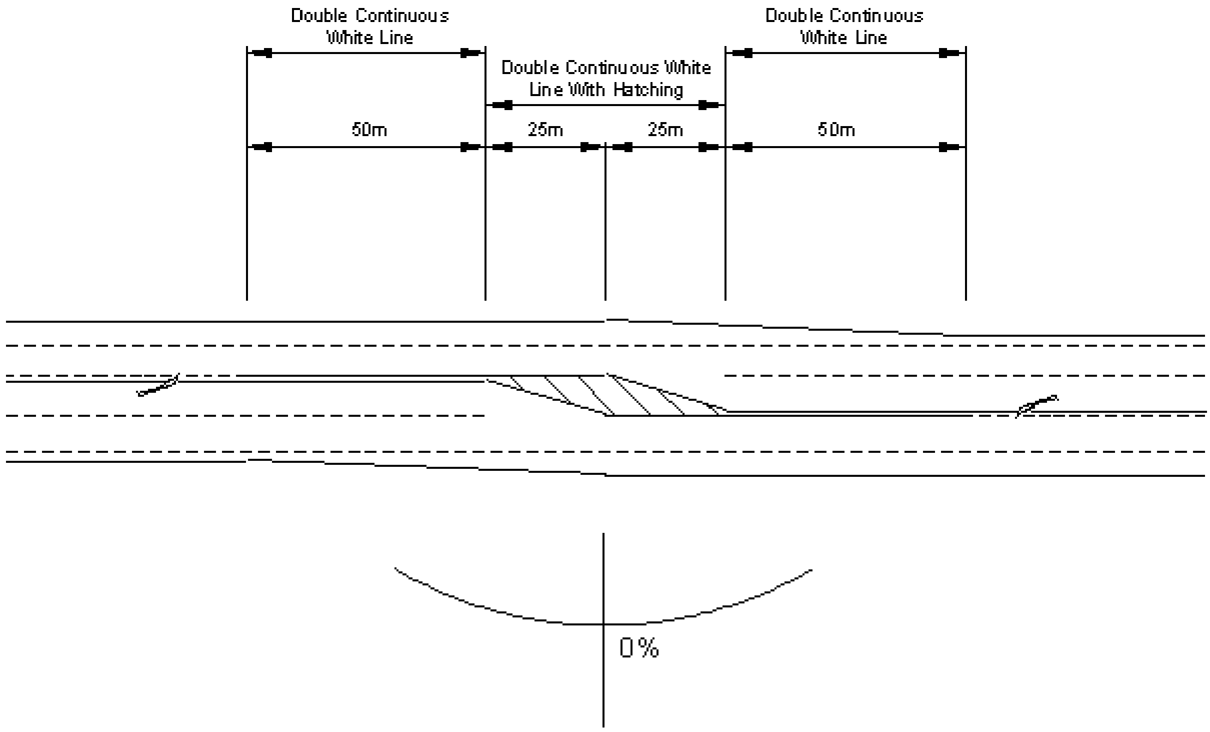


Figure 5/7: Sag Between Two Climbing Lanes

5.35 Marking of Climbing Lanes: A three-lane hill is marked with a lane line separating the two uphill lanes and a double white line separating the uphill lanes from the downhill lane. The double white line will feature a continuous line for uphill traffic in all cases and a continuous line for downhill traffic except where the criteria for a broken line are satisfied (see the Traffic Signs Manual).

5.36 To avoid frequent changes of pattern on long hills, or for safety reasons, the designer may use a downhill continuous line even where the visibility criteria for a broken line are satisfied. However, the use of a prohibitory line on long straight sections should be avoided if possible.

5.37 The markings at the start of the climbing lane should be designed to encourage uphill drivers to keep to the left-hand lane unless overtaking (see Figure 5/4). In order to avoid a potential conflict at this point between uphill and downhill drivers, a length of double continuous line should be provided after (uphill of) the start taper for a distance ‘W’ (see Table 5/1) according to the Design Speed of the road. This ensures that any downhill overtaking vehicle will be returned to its own lane before coming into conflict with an uphill vehicle. In addition, the double white line may be extended along the length of the taper in order to prevent overtaking by downhill traffic. However, if visibility over this length is good, a warning line may be more effective.

Table 5/1: Length of Double Continuous Line

Design Speed (km/h)	Double Continuous Line Length ‘W’ metres
100	245
85	205
70	175
60	145

5.38 Typical layouts for the markings at the end of a climbing lane are indicated in Figures 5/5 and 5/6.

Dual Carriageways and Motorways

5.39 Criteria for Provision: On dual carriageway roads and motorways on hills with gradients (G = 100H/L) greater than 3% and longer (L) than 500m, as an alternative to economic appraisal, a climbing lane should generally be provided wherever the predicted traffic flow in the design year is expected to exceed 75% of the capacity given in Table 4 for the relevant category of road. The height risen (H) and length (L) shall be calculated between two standard points on the hill as illustrated in Figure 5/2.

5.40 Lane Widths: In general, a full lane width climbing lane shall be provided, although in difficult areas, where structural or environmental costs are high, the cross-section may be reduced by using narrow lanes down to 3.25m, i.e. a carriageway width of 9.75m (D2), or 13.00m (D3). Such reductions shall be considered as Departures.

5.41 Provision of Climbing Lanes: On motorways and high quality dual carriageways, climbing lanes shall be formed by the addition of an extra lane, with a full width hard shoulder alongside. On other dual carriageways, a 3.5m wide climbing lane and a 1.0m wide hard strip shall replace the normal hard shoulder. The transition from hard shoulder to hard strip, or vice versa, should take place over the length of the taper in carriageway width.

5.42 Layout at Start of Climbing Lane: The full width of the climbing lane shall be provided at a point 'S' in a similar manner to that described for single carriageway roads (Paragraph 5.23), as shown in Figure 5/8. Wherever possible the additional width should be developed by using the road curvature to provide a smooth transition.

5.43 Climbing lanes may not be inserted directly at the exit from a roundabout, but should allow for a distance of at least 100m before the start of the entry taper to avoid conflicting traffic movements on exiting the roundabout. The entry taper can be reduced to 1/40 owing to the reduced vehicle speeds close to the roundabout.

5.44 Layout at End of Climbing Lane: The carriageway width shall be maintained up to a point F, in a similar manner to that described for single carriageway roads (Paragraph 5.25), followed by a taper of 1/70. A smooth transition should be used wherever possible.

5.45 The climbing lane may terminate at a roundabout where appropriate, with the overtaking lane becoming the right hand entry lane into the roundabout. If the climbing lane would terminate within 500m of the roundabout, it should be continued to the roundabout.

5.46 Signing of Climbing Lanes: To distinguish the commencement of a climbing lane on a dual carriageway from a change of carriageway standard, "Slow Lane" signing should be provided in accordance with the Traffic Signs Manual.

5.47 Sight Distance Requirements with Climbing Lanes: As the speeds of vehicles using the climbing lane will be less than those on the rest of the dual carriageway, the Stopping Sight Distance measured from the centre of the climbing lane may be the distance for one Design Speed step below that for the road. However, the Stopping Sight Distance measured from the centres of the nearside and offside lanes of the original carriageway shall be in accordance with the requirements of Chapter 2 for the Design Speed of the road.

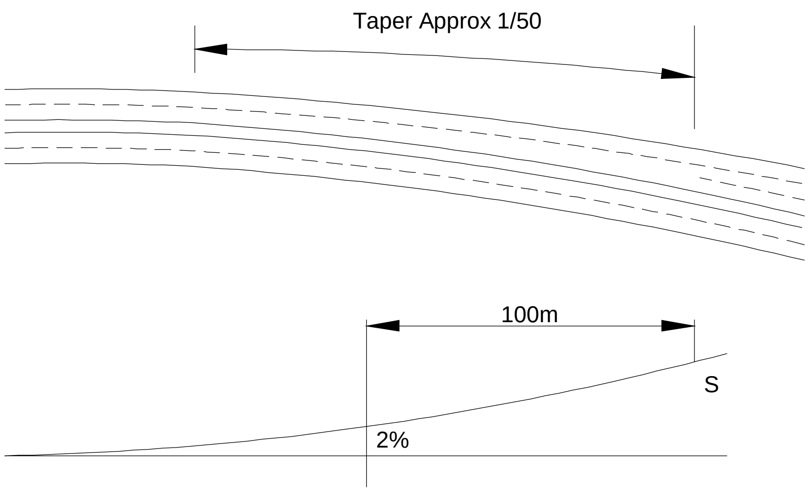
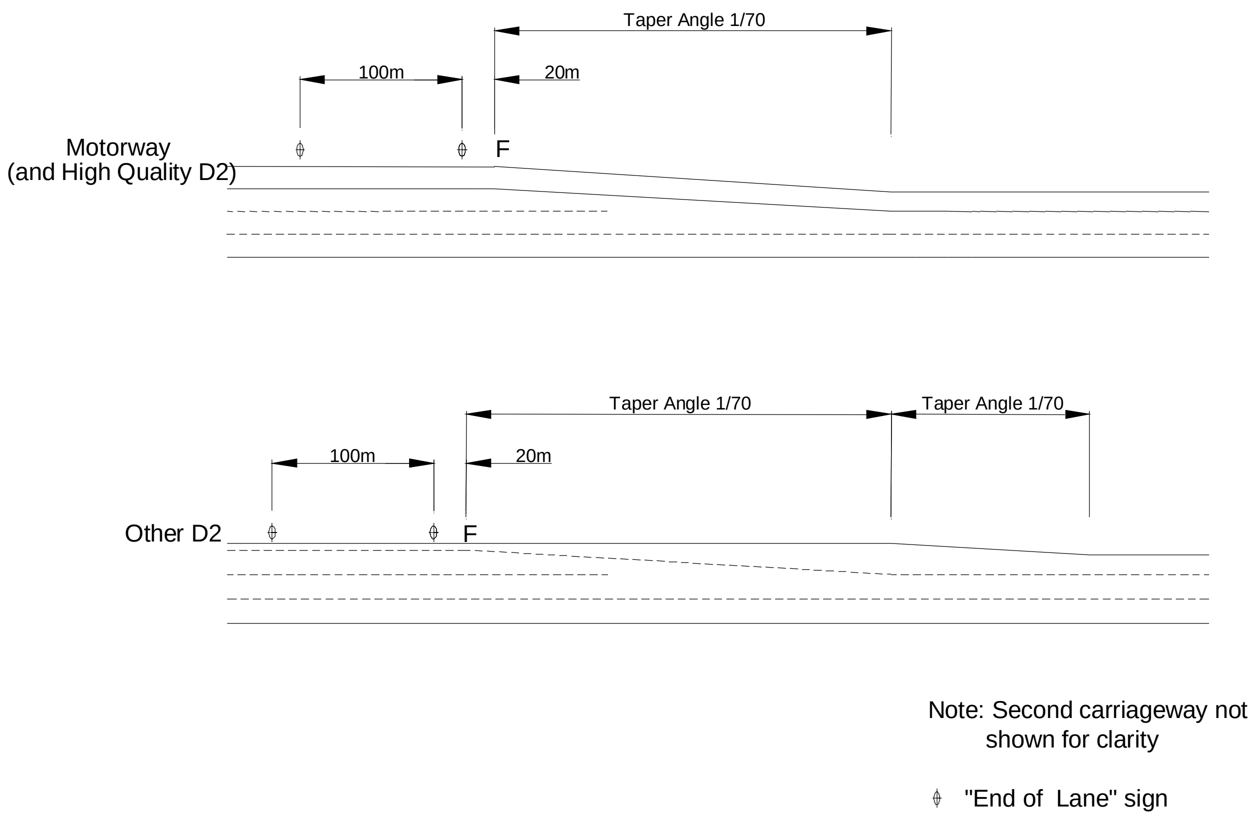


Figure 5/8: Start of Dual Carriageway Climbing Lane



2+1 Roads

5.48 On 2+1 roads a climbing lane consists of the provision of a two-lane 'passing lane' section over the requisite length of road.

5.49 On 2+1 roads on hills with gradients (G = 100H/L) greater than 2% and longer (L) than 500m a climbing lane will normally be justified if the height risen (H) is equal to or greater than the value shown in Table 5/2 for the relevant traffic flow.

Table 5/2: Justification for Climbing Lane on 2+1 Road

Design Year Traffic Flow Two-Way (AADT)	Height Risen (H)
8,000 – 11,000	20m
> 11,000	15m

The height risen (H) and length (L) shall be calculated between two standard points on the hill as illustrated in Figure 5/2.

5.50 Where the above criteria are met, a two-lane uphill section should be provided over the relevant length of the hill between points 'S' and 'F' (see Paragraphs 5.23 and 5.25). The general principles of the climbing lane should be in accordance with the requirements for single carriageways, but the cross-section, road markings and the geometric layout at each end shall be in accordance with the requirements for 2+1 roads (see Chapter 9). Designers should note that, at the end of a two-lane section (including at a climbing lane) on a 2+1 road, the onus is on the overtaking traffic to yield to traffic in the left lane.

5.51 Where the length of the climbing lane on a 2+1 road is such that the one-lane downhill section would be longer than 3,000m, a section of two-lane carriageway should be provided for downhill traffic too (see Paragraph 9.18). This will result in a length of '2+2' road with a cross-section as shown in Figure 9/4. The two-lane downhill section needs to be only long enough to provide a reasonable overtaking opportunity (see Paragraph 9.17).

5.52 Even where there is no need for a climbing lane, the design of 2+1 roads should be coordinated with the vertical alignment so that the two-lane sections function as climbing lanes. Speed differentials between light and heavy vehicles are greater on up gradients, so provision of two-lane sections on up gradients will allow more vehicles to overtake in a given length and will reduce driver frustration. Wherever practicable therefore, two-lane sections should be arranged to coincide with up gradients.

5.53 Even where there is no need for a climbing lane, a two-lane section should not be terminated on a hill with a gradient greater than 2% and longer than 300m. Instead, the two-lane section should be extended to point 'F', 220m beyond the point where the gradient reduces to 2% (see Figure 5/5).

6. INTRODUCTION TO COORDINATED LINK DESIGN

General

6.1 The various elements detailed in this Interim Advice Note shall be coordinated, together with cross-section and junction layouts, so as to ensure that the three dimensional layout as a whole is acceptable in terms of traffic safety and operation, and economic/environmental effects. Single carriageway design is given particular emphasis due to the problems of driver understanding and provision for overtaking.

Rural Roads

6.2 A general guide to the layout features appropriate for various types of road is given in Table 4. The table recommends edge treatments, access treatments and junction types that would be suitable in broad terms for each type of road. For details of the standard road cross-sections, see NRA TD 27.

6.2A The vehicle flows (Annual Average Daily Traffic) given in Table 4 represent the approximate two-way flows which correspond to Level of Service D in reasonably level terrain. This is the level of service at which passing becomes extremely difficult, with 'shock waves' beginning to affect the overall flow. For further details of the level of service, see NRA TA 43.

Urban Roads

6.3 It is not possible to tabulate overall layout characteristics for roads in urban areas in the same way as for rural areas, as the constraints of the existing urban fabric will result in designs tailored to meet the site-specific requirements. Urban Standards (embracing mandatory speed limits, Design Speeds generally 85km/h and below, and reduced cross-section design), are more conducive to safe conditions where the surrounding development is very much of an urban nature. Urban standards should not normally be used for roads which present an open aspect (e.g. passing through parkland, recreational areas, non-built up waste land, etc.), other than for short lengths.

6.4 In urban areas there will usually be less scope for coordinating the geometric features than in rural areas, although wherever economically and environmentally practicable every effort should be made to do so. The demands of accommodating the road within the urban fabric will frequently predominate.

6.5 A single two-lane carriageway S2 or WS2 urban road, with no frontage access, no standing vehicles and negligible cross traffic, would normally represent a radial or orbital bypass or new town distributor. The design considerations in respect of Overtaking Sections in Chapter 7 should be applied to such a road wherever economically and environmentally practicable, although the constraints of the urban area will frequently not permit the flexibility of alignment required. In some cases, extra road width (i.e. WS2 or even a 4 lane single carriageway) can be used to provide overtaking opportunities if economically feasible.

6.6 Single two-lane carriageways S2 or WS2 with frontage development, side roads, bus stops, etc. with the paramount need to create safe conditions for pedestrians, are likely to be modest projects in an area where comprehensive traffic management has been carried out on the existing network and the new road is required to extend or improve that management. It is unlikely that the coordinated design aspects contained hereafter will be applicable in these cases. Further advice is given in the NRA Guidelines on Traffic Calming.

Table 4: Recommended Rural Road Layouts

Category	Type of Road [1.]	Capacity (AADT) for Level of Service D	Edge Treatment	Access Treatment	Junction Treatment at Minor Road	Junction Treatment at Major Road
1	Reduced Single (7.0m) Carriageway S2	8,600	0.5m hard strips. Footways where required	Minimise number of accesses to avoid standing vehicles and concentrate turning movements	Priority junctions, with ghost islands where necessary	Ghost islands
2	Standard Single (7.3m) Carriageway S2	11,600	2.5m hard shoulders	As 1	Priority junctions, with ghost islands where necessary.	Ghost islands or roundabouts [2.]
3	Wide Single [3.] (10m) Carriageway WS2	13,800	2.5m hard shoulders. Pedestrian usage minimised	As 1	Ghost islands. Some side roads stopped up. Occasional bridges at higher end of traffic range	At-grade roundabouts [2.]
4	2+1 Road [4.] (7.0m + 3.5m)	17,250	1.0m and 0.5m hard strips	Minimise number of accesses to avoid standing vehicles and concentrate turning movements.	Restricted number of left in/left out or ghost priority junctions. .	Priority junctions or at-grade roundabouts. Compact grade separation if economically justified.
5A	At Grade Standard Dual 2 Lane (7.0m) Carriageways. All Purpose D2AP	26,500	2.5m hard shoulders	Minimise number of accesses to avoid standing vehicles and concentrate turning movements. No gaps in the central reserve.	Priority junctions. No other gaps in the central reserve.	At-grade roundabouts. Grade separation if economically justified.
5B	Grade Separated Standard Dual 2 Lane (7.0m) Carriageways. All Purpose D2AP	42,000	2.5m hard shoulders	No accesses	Left in/Left out only. No gaps in the central reserve.	Full-grade separation.
5C	High Quality Dual 2 Lane (7.0m) Carriageways. All Purpose D2AP	52,000	2.5m hard shoulders	No accesses	None	Motorway standards.
6A	Wide Dual 2 Lane (7.5m) Carriageways. All Purpose D2AP	44,100	3m hard shoulders	Minimisation of access numbers severely enforced. No gaps in the central reserve.	Restricted number of priority junctions. No other gaps in the central reserve.	At grade roundabouts at lower end of range. Otherwise full grade separation.
6B	Grade Separated Wide Dual 2 Lane (7.5m) Carriageways All Purpose D2AP	44,100	3m hard shoulders	No accesses	Left in/Left out only. No gaps in the central reserve.	Full grade separation.
7A	Standard Dual 2 Lane (7.0m) Motorway D2M	52,000	2.5m hard shoulders	Motorway Regulations	None	Motorway standards
7B	Wide Dual 2 Lane (7.5m) Motorway D2M	55,500	3m hard shoulders	Motorway Regulations	None	Motorway standards

Notes:
1. For details of the standard road cross-sections, see NRA TD 27.
2. Single lane dualling may be appropriate in some situations, but would be a Relaxation (see TD 42).
3. The approval of the National Roads Authority is required for schemes which will create more than 2km of Wide Single Carriageway.
4. The approval of the National Roads Authority is required for schemes incorporating 2+1 roads.

7. SINGLE TWO-LANE CARRIAGEWAY ROADS

General Principles

7.1 Single two-lane carriageway roads up to 10m wide (running width) shall be designed with the objectives of safety and uncongested flow in mind. This Chapter gives methods of achieving those objectives. Although they are to some extent related, for instance frustrated traffic tends to lead to unsafe conditions, it is important to identify other aspects which, if not taken into account in the design, may lead to a higher than average proportion of serious accidents. Amongst these are:

a) Continuous flowing alignments, (Paragraphs 7.25 and 7.28);
b) Treatment of grade separation on single carriageways (Paragraph 7.35);
c) Single carriageway alternating with dual carriageway (Paragraphs 7.16, 7.36, 7.39, 7.40 and 7.41);
d) Staged construction (Paragraphs 7.37, 7.38, 7.47 and 7.48).

7.2 Clearly identifiable Overtaking Sections for either direction of travel are to be provided frequently throughout the single carriageway, so that vehicles can maintain the Design Speed in off-peak conditions. In peak conditions overtaking opportunities will be rare; nevertheless steady progress will be possible for the majority of vehicles if junctions are carefully designed, and if climbing lanes are provided wherever the forecast traffic demand is sufficient to justify a climbing lane in accordance with Chapter 5.

7.3 In easy terrain, with relatively straight alignments, it may be economically feasible to provide for continuous overtaking opportunity by means of consistent provision of Full Overtaking Sight Distance (FOSD). Where significant curvature occurs or the terrain becomes increasingly hilly, however, the verge widening and vertical crest requirements implicit in this design philosophy will often generate high cost and/or environmentally undesirable layouts. The alternative philosophy of clearly identifiable Overtaking Sections, including climbing lanes, interspersed with clearly non-overtaking sections, will frequently result in a more cost effective design provision. The trade-off between the construction and user costs, including accidents, should be tested for alternative alignments by cost/benefit analyses.

7.4 In the coordination of vertical and horizontal alignments, many of the principles contained in Paragraph 8.7 (Categories 5B and 6B dual carriageways) are equally applicable to the design of single carriageway roads. However, the overriding need to design for adequate overtaking will frequently supersede the general desirability for full coordination of vertical and horizontal alignments, with design concentrating upon the provision of straight Overtaking Sections. Nevertheless, designs should still be checked at sags and crests to ensure that the road in perspective does not take on a disjointed appearance.

Overtaking Sections

7.5 Overtaking Sections are sections of road where the combination of horizontal and vertical alignment, visibility, or width provision is such that clear opportunities for overtaking will occur. Overtaking Sections, which are fully defined in Paragraphs 7.7 to 7.16, comprise:

a) Two-lane Overtaking Sections;
b) Climbing Lane Overtaking Sections;
c) Downhill Overtaking Sections at Climbing Lanes;
d) Dual or Single 4-lane Overtaking Sections.

It is necessary for the calculation of Overtaking Value (see Paragraph 7.20) to define the method by which the lengths of Overtaking Sections are assessed, and the method of measurement for each category of Overtaking Section is described in the following paragraphs. In general, Overtaking Sections will commence whenever either FOSD on a straight (or nearly straight) or right hand curve is achieved, or the width provision is sufficient for overtaking without crossing the dividing line between opposing lanes. They will terminate either at a point where sight distance reduces to FOSD/2 when approaching a non-overtaking section, or at a distance of FOSD/4 prior to an obstruction to overtaking (the detailed measurement of single lane downhill sections opposite climbing lanes, however, is described in Paragraph 7.13).

7.6 The method of measurement described in the following paragraphs is based upon curvature/visibility relationships for S2 roads. Whilst the additional road width of a WS2 provides much greater flexibility for overtaking, largely independent of curvature, the following design rules should still be used to achieve an optimal overtaking design.

7.7 Two-lane Overtaking Sections: Two-lane Overtaking Sections are sections of single two lane carriageways, with normal centre of carriageway road markings providing clear opportunities for overtaking. They consist of straight or nearly straight sections affording overtaking in both directions (with horizontal radius of curvature greater than that shown in Table 5) and right hand curves, the commencement of which are provided with at least FOSD. The section, which is shown in Figure 19, is measured as follows:

7.8 Commencement: At the point on a straight (or nearly straight) or right hand curve where FOSD is achieved, either within or without the road boundary.

7.9 Termination:

a) At a point FOSD/4 prior to the tangent point or centre of transition of a left hand curve; or
b) The point on a right hand curve where sight distance has reduced to FOSD/2; or
c) A point FOSD/4 prior to an obstruction to overtaking (see Paragraph 7.18).

Table 5: Minimum Radii for Two-lane Overtaking Sections

Design Speed km/h	100	85	70	60	50
Minimum Radius of Straight or nearly Straight sections (m)	8160	5760	4080	2880	2040

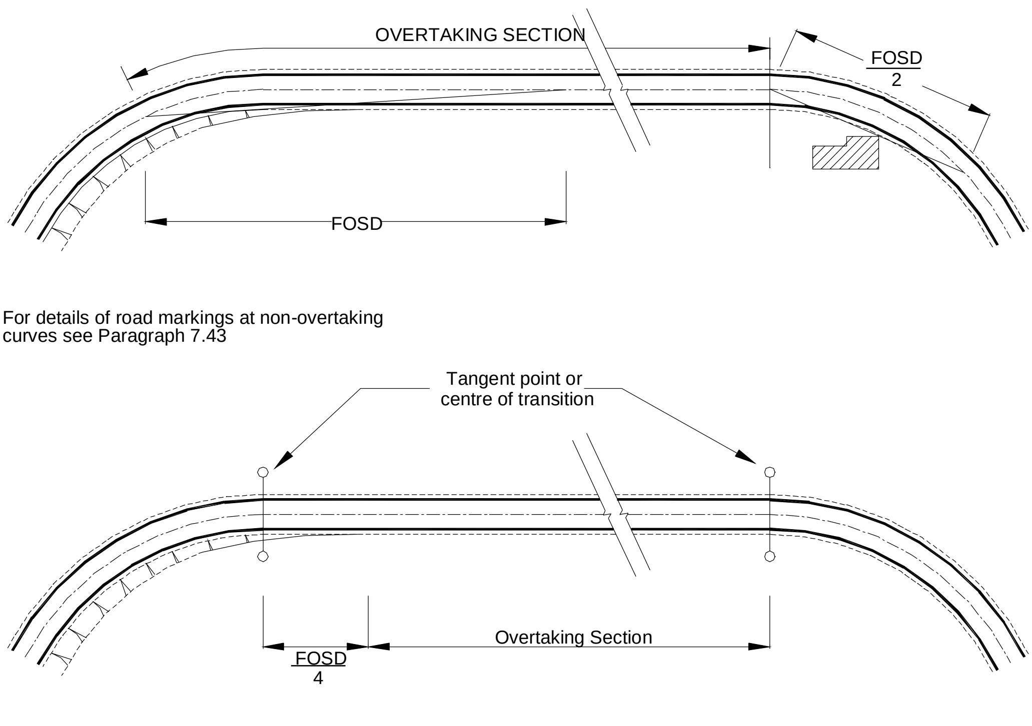


Figure 19: Two-lane Overtaking Sections

7.10 Climbing Lane Overtaking Sections: Climbing Lane Overtaking Sections are sections where priority uphill overtaking opportunities are provided by means of two uphill lanes, separated from the opposing downhill lane by means of a double line, (either double continuous or continuous/broken). The section, which is shown in Figure 20, is measured as follows:

7.11 Commencement: A point in the centre of the commencing taper.

7.12 Termination: A point FOSD/4 prior to the centre of the finishing taper. However, if the following section is an Overtaking Section, it should be assumed to be contiguous with the climbing lane section.

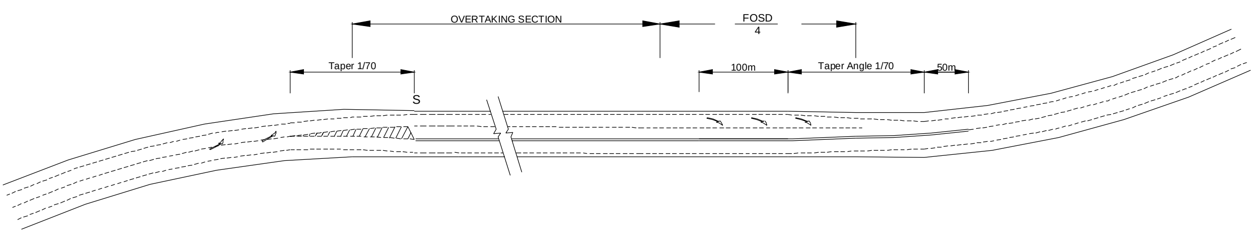


Figure 20: Climbing Lane Overtaking Sections

7.13 Downhill Overtaking Sections at Climbing Lanes: Downhill Overtaking Sections at Climbing Lanes are sections of a single downhill lane, opposite a climbing lane, constrained by a continuous/broken double line, where the combination of visibility and horizontal curvature provides clear opportunities for overtaking when the opposing traffic permits. They consist of straight or nearly straight sections, and right hand curves with radii greater than those shown in Table 6.

Table 6: Minimum Radii of Right Hand Curves for Downhill Overtaking Sections at Climbing Lanes

Design Speed km/h	100	85	70	60	50
Minimum Radius m	2880	2040	1440	1020	720

The sight distance naturally occurring within the normal road boundaries at the radii shown in Table 6 will be sufficient for downhill overtaking, and thus, for Downhill Overtaking Sections at Climbing Lanes, verges shall not be widened to give FOSD. However, these sections should only be considered as Overtaking Sections on straight grades or sag configurations, or when the crest curve K value is large enough that the road surface is not obscured vertically within FOSD – this will require the use of a crest curve K value of double the value given in Table 3 for FOSD Overtaking Crest K Value.

The section, which is shown in Figure 21, is measured as follows:

7.14 Commencement: The point where the right hand curve radius achieves the requisite value from Table 6.

7.15 Termination: A point FOSD/4 prior to the end of the requisite radius or a point FOSD/4 prior to the centre of the finishing taper, whichever is the earlier.

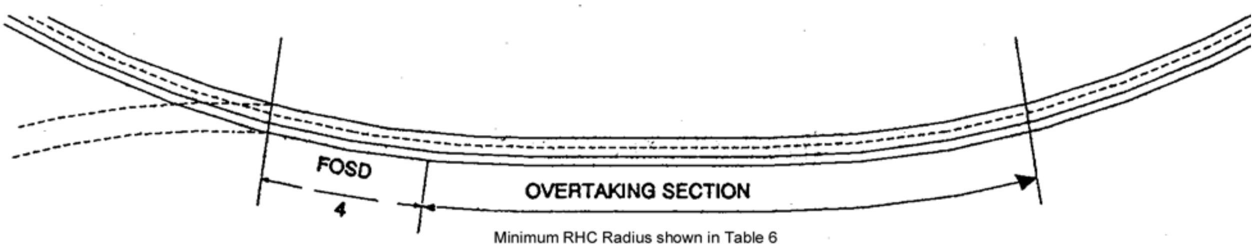


Figure 21: Downhill Overtaking Sections at Climbing Lanes

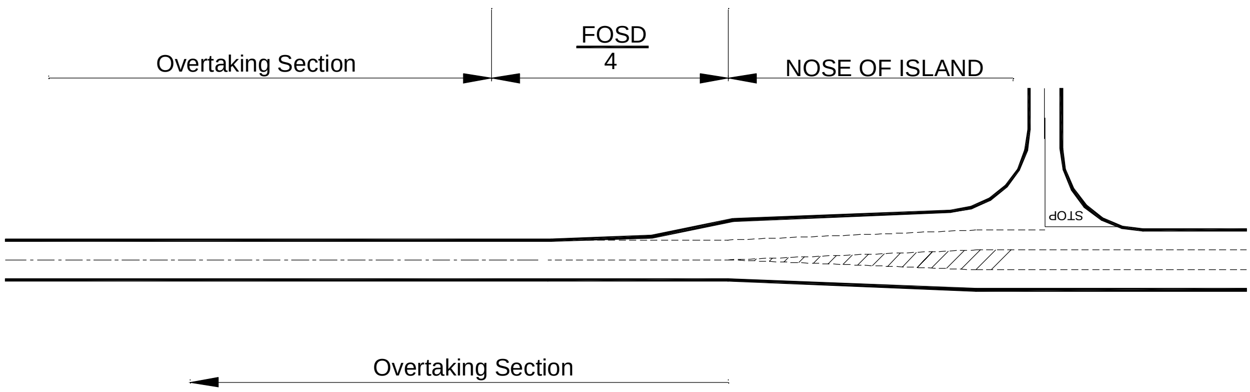
7.16 Dual Overtaking Sections: Dual Overtaking Sections are sections with dual carriageways, which provide overtaking opportunities throughout their length. They should, however, only be provided in cases where the most economic method of improvement of a section of existing single carriageway is to provide a second carriageway alongside the first. Dual Overtaking Sections within otherwise single carriageway roads shall be subject to the same overtaking length criteria as climbing lane sections shown at Paragraph 7.10. Single 4-lane Overtaking Sections (where space is limited) should be considered equivalent to Dual Overtaking Sections in terms of assessment of overtaking.

Non-overtaking Sections

7.17 Non-overtaking Sections are all lengths of single carriageway roads that do not conform with the requirements of Paragraphs 7.7 to 7.16. These will generally be left or right hand curves on two-lane sections, single downhill lanes opposite climbing lanes, or approaches to junctions (see also Non-overtaking crests, Paragraph 7.19).

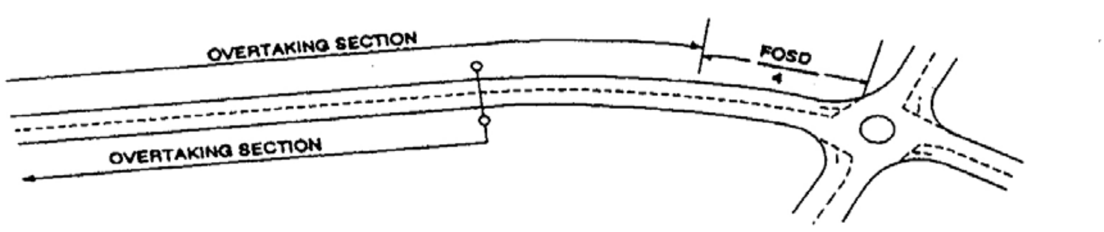
Obstructions to Overtaking

7.18 At Grade Junctions: Major/minor junctions with ghost islands or single lane dualling and roundabouts should be considered as obstructions to overtaking if they are sited within an otherwise Overtaking Section. The Overtaking Section shall terminate at a distance of FOSD/4 prior to the nose of the ghost or physical island, or the roundabout Yield line, as shown in Figure 22. Similarly, the Overtaking Section shall commence at the end of the nose of the ghost or physical island at a priority junction. The commencement at a roundabout shall be in accordance with the requirements for a Two-lane Overtaking Section (see Paragraph 7.8). However, simple junctions and accesses with no central ghost or physical islands can be ignored for the purpose of determining Overtaking Sections.



Note: a simple priority junction with no ghost island layout can be ignored for the purposes of determining Overtaking Sections.

(a) Approach to Priority Junction (with ghost or solid island).



(b) Approach to Roundabout.

Figure 22: Obstructions to Overtaking: At Grade Junctions

Non-overtaking Crests

7.19 A crest with a K value less than that shown in Table 3 for FOSD Overtaking Crest K Value should be considered as a Non-overtaking crest. The Overtaking Section within which it occurs should be considered to terminate at the point at which sight distance has reduced to FOSD/2, as shown in Figure 23. However, when the horizontal alignment of the Overtaking Section is straight or nearly straight, the use of Desirable Minimum crest K values would result in a continuous sight distance only slightly above FOSD/2, and thus, theoretically, the Overtaking Section would be continuous over the crest. The use of crest K values greater than Desirable Minimum but less than FOSD Overtaking Crest in combination with a straight or nearly straight horizontal alignment (such that the section of road could form part of a Two-lane Overtaking Section in the horizontal sense) is not, therefore, recommended for single carriageway design (see Paragraph 7.30), and is considered to be a Departure from Standards. An exception to this is on the approach to a junction: it is important for Desirable Minimum Stopping Sight Distance to be provided at the junction, so the requirements of Paragraph 1.28 take precedence.

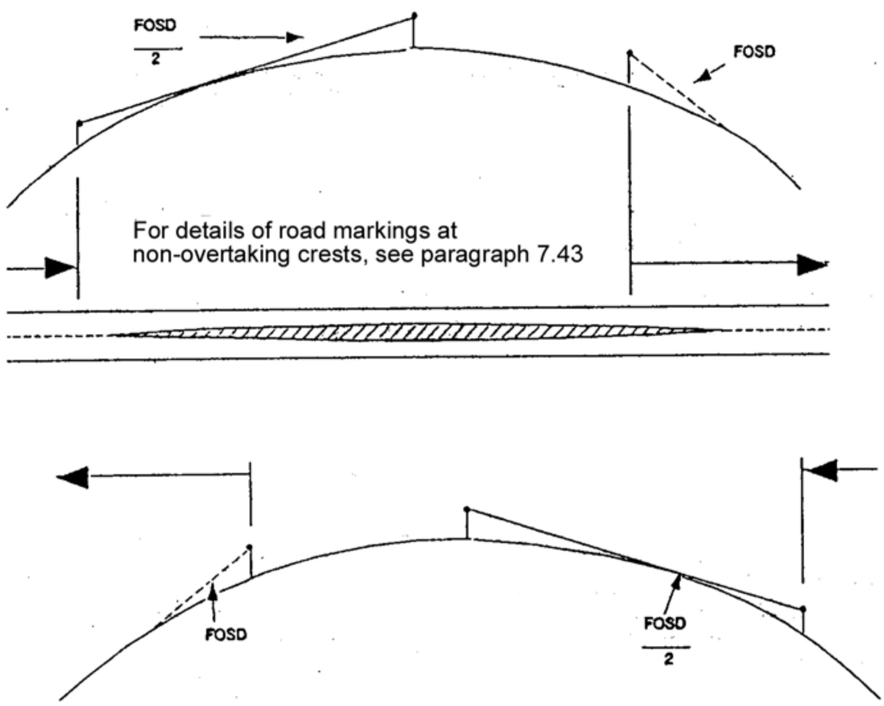


Figure 23: Non-overtaking Crest

Overtaking Value

7.20 On Rural Roads, a sight distance analysis shall be carried out for each direction of travel to ensure that there are sufficient and effective Overtaking Sections at frequent intervals along the scheme. The total length of Overtaking Sections for each direction shall be summed and divided by the total length of the road improvement to obtain the "Overtaking Value" in each direction, expressed as a percentage. The minimum Overtaking Values for the different road types which are thought to provide a reasonably safe road in most circumstances are given in Table 7. An Overtaking Value is not required on single carriageway Urban Roads.

Table 7: Overtaking Value

Rural Road Type	Overtaking Value
Reduced Single	15%
Standard Single S2	30%
Wide Single WS2	40%

The table applies to new construction and new schemes exceeding 2km. Overtaking sections should be distributed along a length of road such that no Non-overtaking Section exceeds 3km. The results of the sight distance analysis should be plotted on the engineering drawings, with the system of road markings to be adopted along the route included below the plot (see Paragraphs 7.7, 7.10, 7.13, 7.19, 7.29, 7.30 and 7.43). This is to ensure that the significance of the various interacting parameters has been taken account of at an early date. Generally speaking it is an advantage from a safety point of view to provide as much overtaking distance as possible, but the amount of provision above the minimum in each scheme must be a matter of judgement according to the particular circumstances.

7.21 The Overtaking Sections along a scheme, which may comprise combinations of the various types shown in Paragraphs 7.5 to 7.16, should be provided by the most economic means. In some instances it may be suitable to use a few long sections, whilst in other cases more frequent shorter sections, linked with Non-overtaking Sections, would provide the most economic strategy to achieve the appropriate Overtaking Value. Alternative designs should be tested by cost benefit analyses.

7.22 The Overtaking Values shown shall be regarded as a minimum level of provision. Using the principles described in this standard it should be possible in the majority of cases to obtain these values without significant extra expenditure on alignment. Detailed guidance is given in Paragraph 7.24. It must be appreciated, however, that a single carriageway will never provide an equal "level of service" to a dual carriageway. There will always be greater interactions between fast and slow moving vehicles on single carriageways, and overtaking manoeuvres will always be hazardous, involving difficult decisions by drivers, whereas dual carriageways permit continuous overtaking without interference with opposing traffic. These implications, however, result in reduced speeds and increased accident rates on single carriageways that are already implicit in the cost/benefit trade-off of alternative standards of design, although the "level of service" or driver-comfort differentials cannot be costed. Provided the requisite Overtaking Values are achieved, therefore, a satisfactory single carriageway design will result. Any additional measures to increase Overtaking Values beyond the requisite levels, such as the provision of additional climbing lanes, straightening route sections, or elimination of junctions, should be justified in economic and environmental terms.

7.23 Schemes Less Than 2km in Length: Schemes less than 2km in length shall be integrated with the contiguous sections of existing road to provide the best overtaking opportunities that can economically be devised. Where contiguous sections afford little or no overtaking opportunity, it is essential that the requisite Overtaking Value be achieved for the scheme. On short improvement schemes this will result in the need to provide at least one Overtaking Section in either direction. However, where contiguous sections provide good overtaking opportunities, a check on the Overtaking Value for a length of, say, 3km including the improvement scheme may relieve the necessity to provide the requisite Overtaking Value for the improvement.

7.24 Means of Improving Overtaking Value: As well as ensuring sufficient overtaking opportunities, the design method outlined above also controls the spacing of junctions. If the criteria are not met initially for any alignment it may be necessary to:

a) Modify the junction strategy by stopping up, bridging or diverting some side roads;

b) Adjust the alignment to produce more straight sections;

c) Introduce climbing lanes on hills previously not considered justified because of low traffic flow;

d) Introduce roundabouts at the more heavily trafficked priority junctions to create sharper changes of direction and improve Overtaking Section lengths;

e) On lengths of existing road without hard shoulders, introduce lengths of Standard S2 or WS2 road with hard shoulders at suitable locations. Whilst this will not improve the Overtaking Value according to the formal methods described in Paragraphs 7.5 to 7.16, such sections will nevertheless, by the extra road width, increase flexibility and reduce frustration; or

f) Introduce more extensive sections of single 4-lane, 2+1 road or dual carriageway.

Alternative means of improving Overtaking Values should be tested by cost/benefit analyses to determine their economic implications. This will take into account any changes in user costs due to increased junction delays, diversion costs, or increased speeds due to increased road width, etc. The minimum overall additional cost of improving Overtaking Values in terms of loss of Net Present Value (NPV) should be identified, and an assessment made taking all factors into account, including the effect on the road user.

The extra cost of provision of extra road width to provide a climbing lane at a hill previously considered unjustified (or a section of wider road cross-section on a constrained level road alignment) may be justified on the total balance of advantage. As the wider road will also provide some improved benefits, the resulting loss of NPV may only be minor and thus a small price to pay for the unquantifiable benefits to traffic of improving the Overtaking Value.

Horizontal Curve Design

7.25 The use of mid-large radius curves is counter productive, inhibiting the design of clear Overtaking Sections. such curves produce long dubious overtaking conditions for vehicles travelling in the left hand curve direction, and simply reduce the length of overtaking straight that could otherwise be achieved. Figure 24 shows a curve selection chart for horizontal curves which illustrates the bands of radii (relative to Design Speed) and their applicability to the design of single carriageways.

7.26 Wherever possible, Overtaking Sections (including climbing lanes) should be provided as straight or nearly straight sections (Band A), thus providing an Overtaking Section for both directions of travel ($V^2/R < 1.25$).

7.27 Where straight sections are not possible, lower radii (Band B) will result in right hand curve (RHC) Overtaking Sections:

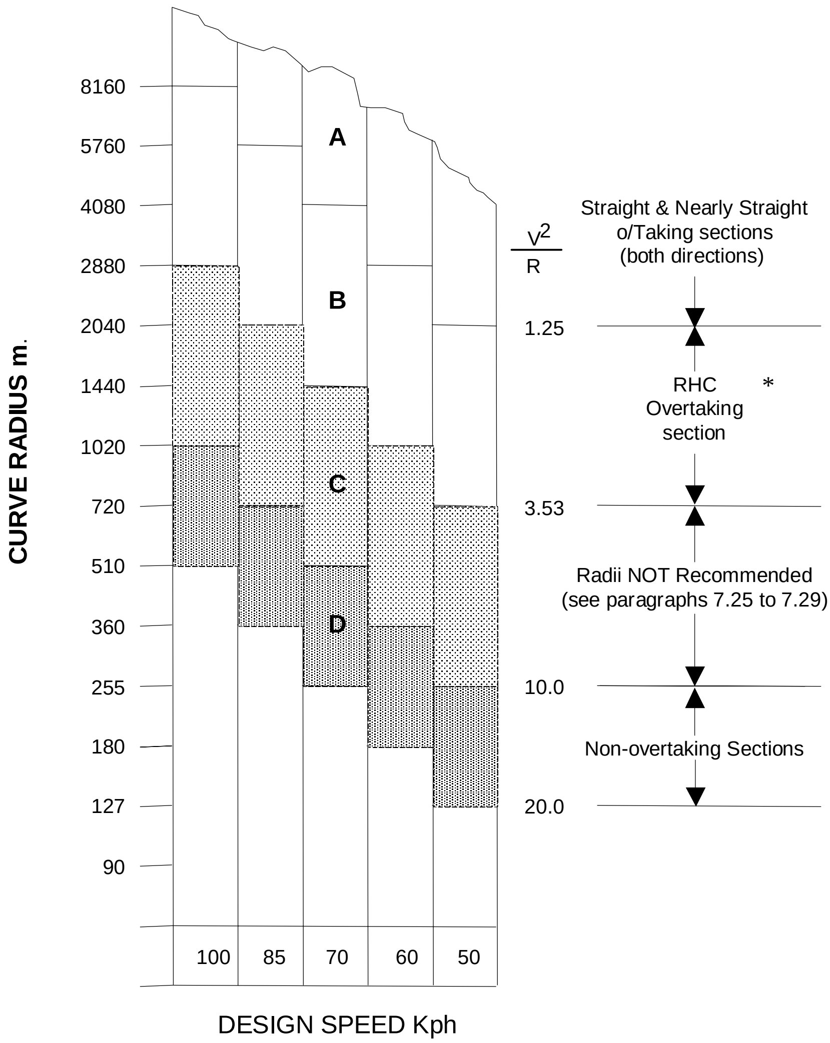
a) On two-lane sections following the achievement of FOSD (see Figure 19); and

b) On single lane downhill sections opposite climbing lanes (see Figure 21).

The lower limit of Band B ($V^2/R = 3.53$) shown for RHC Overtaking Sections should be considered as the minimum radius for use in designing Overtaking Sections. At this level a maximum verge width of 8.45m (plus the 2.5m hard shoulder) would be required on a Standard Single Carriageway to maintain FOSD within the road cross-section for RHC traffic. Left hand curves with radii in Band B should not be considered to be part of Two-Lane Overtaking Sections or Downhill Overtaking Sections at climbing lanes.

7.28 The use of radii in Band C ($3.53 > V^2/R < 10$) is not recommended, as they, in common with Band B, provide long sections with dubious overtaking conditions for LHC traffic. Where visibility is constrained within the road cross-section, either excessive verge widening would be required to maintain FOSD for RHC traffic, or the natural visibility without verge widening at these radii would result in dubious overtaking conditions. It is a paramount principle, therefore, that design should concentrate only on Bands A and B for clear Overtaking Sections, and Band D for clear Non-overtaking Sections. The use of radii in Band C is a Departure from Standards (see Paragraph 1.31).

7.29 Non-overtaking Sections should be designed using the radii shown in Band D (V^2/R = 10 to 20), where the radius is sufficiently small to represent a clearly Non-overtaking Section. Radii of Non-overtaking Sections should be chosen around the centre of Band D (V^2/R = 14) to strike a balance between providing clear Non-overtaking Sections and avoiding steep superelevation.



* Note: Verge widening may be necessary. See Paragraph 7.27.

Figure 24: Horizontal Curve Design

Vertical Curve Design

7.30 The vertical alignment shall be coordinated with the horizontal alignment to ensure the most efficient overtaking provision. On Two-Lane Overtaking Sections, the vertical curvature shall be sufficient to provide for FOSD in accordance with Paragraphs 2.3 to 2.5. However, for Non-overtaking Sections and climbing lanes, the use of large crest curves is quite unnecessary and is not recommended. On a road with a straight or nearly straight horizontal alignment, the use of a crest curve that was large but not sufficient to provide FOSD would result in a long section of dubious visibility (see Paragraph 7.19). Therefore, the following standards shall apply for crest curves on single carriageway roads with a straight or nearly straight horizontal alignment (such that the section of road could form part of a Two-lane Overtaking Section in the horizontal sense):

a) Unless FOSD is provided, the crest K value should not be greater than that for one Design Speed step below Desirable Minimum;

b) The use of crest K values greater than one Design Speed step below Desirable Minimum and up to Desirable Minimum is not preferred, but may be used as a Relaxation;

c) The use of crest K values greater than Desirable Minimum but less than FOSD Overtaking Crest is not recommended and is considered to be a Departure from Standards. The use of crest curves in that range would be counter productive, increasing the length of dubious crest visibility, and reducing the length of clear Overtaking Sections that could otherwise be achieved;

d) Notwithstanding (a) to (c) above, the crest curve K value on the immediate approaches to junctions shall be not less than the Desirable Minimum, in accordance with Paragraph 1.28.

7.31 Horizontal and vertical visibility shall be carefully coordinated to ensure that sight distance at curves on crests is correlated. For example, it would be unnecessary to acquire additional verge width to provide for Desirable Minimum Stopping Sight Distance in the horizontal sense, when the crest only provides a Stopping Sight Distance of one Design Speed step below Desirable Minimum.

7.31A On sections of road where the alignment allows overtaking, “blind spots” must be avoided. These occur when the road disappears from view over a crest or around a bend and reappears in view again further on. Vertical blind spots, or “hidden dips” occur where there is a sag between two crests on a straight road; horizontal blind spots occur where reverse horizontal curves are used on a straight grade. These, plus a combination of horizontal and vertical geometry, could cause the road to disappear from view such that a car coming around a bend or over one crest can see the road ahead (on the far crest) but may not be able to see an oncoming car in the intervening space. As blind spots can be the cause of overtaking accidents, FOSD must be provided both horizontally and vertically in each direction of travel on these sections of road.

Junction Strategy

7.32 The aim should be to provide drivers with layouts that have consistent standards and are not likely to confuse them. On lengths of inter-urban road, sequences of junctions should not therefore involve many different layout types. For example, a length of route containing roundabouts, single lane dualling, ghost islands, simple priority junctions and grade separation would inevitably create confusion and uncertainty for drivers and cause accidents on that account. The safest road schemes are usually the most straightforward ones that contain no surprises for the driver.

7.33 Major/minor junctions with ghost islands or local single lane dualling and roundabouts represent an obstruction to overtaking. To achieve maximum overtaking efficiency, therefore, straight Overtaking Sections should be located wherever possible between junctions, which can be located in Non-overtaking Sections. Visibility to the junction shall be a minimum of Desirable Minimum stopping sight distance.

7.34 Use of a roundabout will enable a change of alignment at a junction, thus optimising the Overtaking Sections either side. As an alternative to continuing large radius curves into the roundabout with only unidirectional overtaking, it is preferable to utilise a straight section followed by a non-overtaking radius as the final approach, in order to optimise the use of bi-directional overtaking straights, as shown in Figure 25.

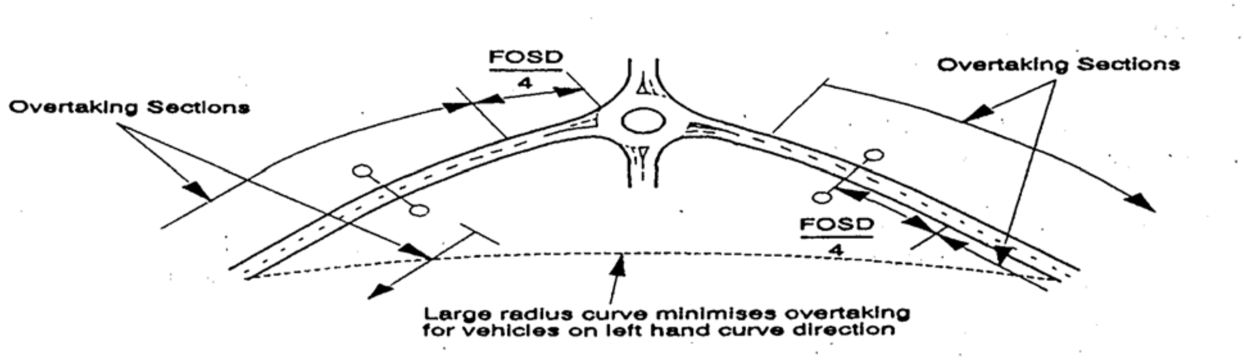


Figure 25: Use of Roundabout to Change Alignment

7.35 Designs involving grade separation of single carriageway roads should be treated with caution. Some grade separated crossings will be necessary for undesirable side road connections and for agricultural purposes. Experience has shown that frequent overbridges and the resulting earthworks create the impression of a high speed road, engendering a level of confidence in the road alignment that cannot be justified in single carriageways, where opposing traffic travels on the same carriageway. The provision of regular at-grade junctions with ghost islands, local dualling or roundabouts will maintain the impression of a single carriageway road. Where crossing flows are high, or local topographical conditions would suggest the need for a grade separated junction, the single quadrant link with a conventional ghost island junction, as shown in Figure 26, will maintain the impression of a single carriageway road, with conventional single carriageway turning movements. This layout can also minimise the disruptive right turn movement onto the major road: the link should be located in the quadrant that will ensure the larger turning movements become left turns onto and right turns off the major road. With the highest levels of traffic flow, it may be necessary to provide roundabouts at one or both ends of the link road. The use of slip merges with acceleration lanes can be confusing on single carriageways and create problems with merging into a single lane. They destroy the overall impression of a single carriageway, and shall not be used.

Changes in Carriageway Width

7.36 Changes from dual to single carriageways are potential hazards: the aim in new construction should be to provide continuity of road type, either single or dual carriageway or 2+1 layout, on any major section of a route which carries consistently similar traffic, subject to satisfactory economic and environmental assessments. Exceptions are described below:

Where it is not possible to achieve an adequate Overtaking Value by means of Two-lane Overtaking Sections or climbing lanes, the impression of a single carriageway road shall be maintained by utilising Standard S2 or WS2 sections with hard shoulders at suitable locations (see Paragraph 7.24), rather than introducing sections of dual carriageway. Alternatively, the use of a 2+1 road could be considered (see Chapter 9) over a reasonable length.

Where it is appropriate to change from dual to single carriageway, careful consideration should be given to the use of a roundabout as a terminal junction to indicate to drivers the significant change in road standard. Whatever layout is adopted, adequate advance signing will be required.

For changes from single carriageways to 2+1 roads, see Paragraph 9.40.

7.37 Single carriageways of a type containing wide verges and extensive earthworks prepared for eventual dualling create the illusion of driving on a dual carriageway: this leads to abnormally high serious accident rates. Where staged construction is part of the design, or where there are safety problems at existing sites, provision shall be made to avoid giving drivers an illusion that they are on a dual carriageway rather than on a single carriageway. Appropriate measures are:

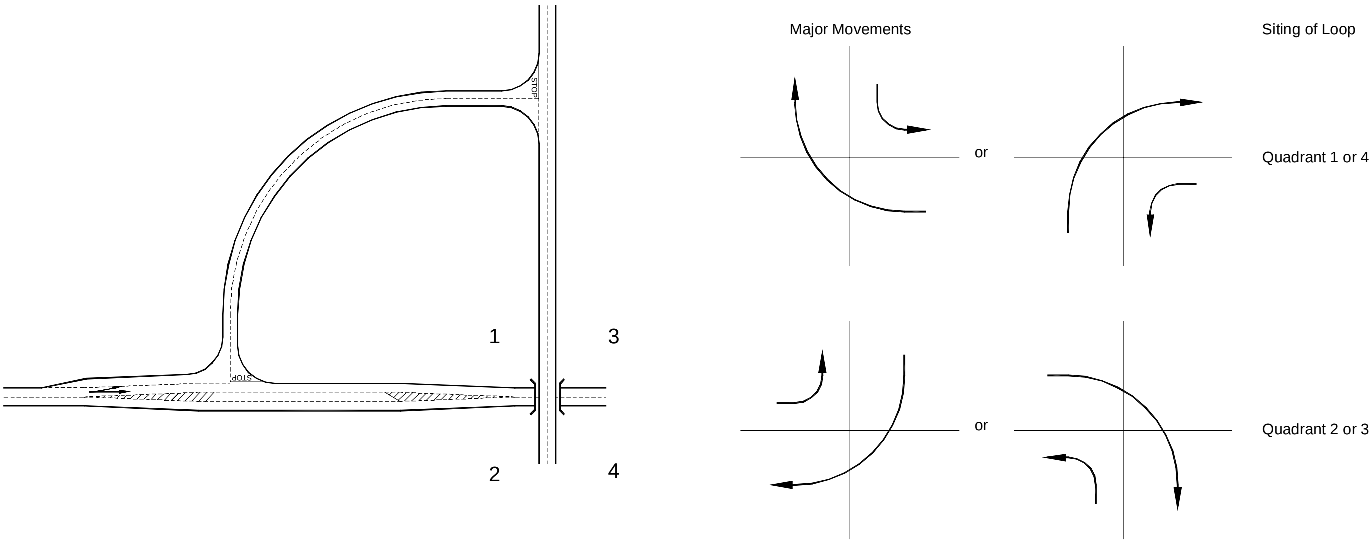


Figure 26: Single Quadrant Link

a) Fencing of a permanent appearance at a verge width (normally 3.0m) from the channel of the constructed carriageway on the side reserved for the future carriageway;

b) Clear signing and marking indicating the existence of two way traffic; or

c) Where a changeover from dual to single carriageway or 2+1 road occurs at a roundabout, provision of a narrow physical splitter island not less than 50 metres long on the single carriageway side of the roundabout followed by hatching.

7.38 Where there is an overbridge designed for an eventual second carriageway, the illusion of a second running carriageway shall be removed by planting and earth mounds as shown in Figure 27.

Figure 27

7.39 Where a lighter trafficked bypass occurs within an otherwise dual carriageway route, a single carriageway or 2+1 road may be acceptable provided the terminal junctions such as roundabouts give a clear indication to drivers of changed standards (see Figure 28 and Paragraphs 7.36 and 7.37 b and c).

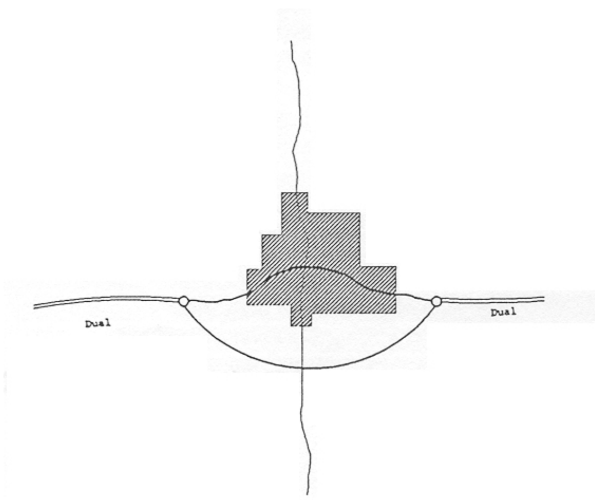


Figure 28

7.40 In circumstances where a length of new carriageway alongside an existing single carriageway provides the most suitable and economic means of achieving a dualled Overtaking Section and where such a dual carriageway returns to single carriageway width or in any other case, the change in width shall be made abundantly clear to drivers by:

a) Signing and marking indicating the existence of the single carriageway; and

b) Providing a length of central reserve in advance of the taper such that drivers approaching the single carriageway can see across it, to have a clear view of the approaching traffic moving onto the dual carriageway.

7.41 If lengths of dual carriageway within a generally single carriageway road or vice-versa are unavoidable, they shall be at least 2km in length and preferably 3km, and major/minor junctions shall be avoided within 1 kilometre of the end of the central reserve on either type of carriageway.

Road Markings

7.42 (Not used.)

7.43 At non-overtaking horizontal curves and crests (see Paragraph 7.30), double continuous line markings should be provided where the visibility (measured in the same way as for FOSD, see Paragraph 2.4) is less than the relevant distance stated in the Traffic Signs Manual. The markings may be strengthened with a hatched marking in accordance with the Traffic Signs Manual, as shown in Figure 29, especially following Overtaking Sections, in order to make clear to drivers the presence of undesirable overtaking conditions.

Existing Single Carriageway Improvements

7.44 The design standards contained in the preceding paragraphs apply generally to lengths of new single carriageway construction, from short bypasses and diversions to extensive new single carriageway routes. When dealing with existing rural roads, the need for improvements will frequently be dictated by evident dangerous bends, junctions, narrow sections, hills, etc.: for the improvement of such features the standards shown in Chapters 1 to 5 should be applied.

7.45 Where, however, the need for improvement arises from congested conditions, or from a restricted alignment providing an unsatisfactory regime of flow, attention should be focused upon the provision of adequate Overtaking Sections, as in Paragraphs 7.20 to 7.24. One of the most economic methods of improving Overtaking Value is the provision of climbing lanes (or a second carriageway added to the first) on hills, where slow moving vehicles create severe congestion and consequent delays. This can be considerably more economic than a major realignment to create a Two-Lane Overtaking Section elsewhere.

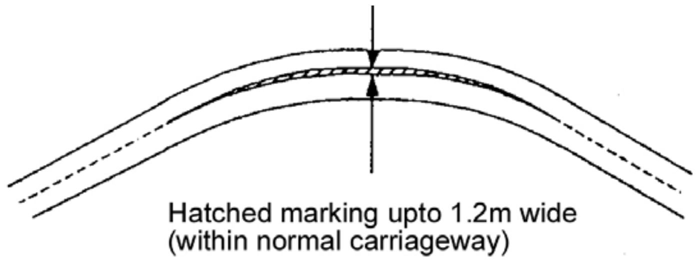


Figure 29: Hatched Road Marking at Non-overtaking Curves and Crests

7.46 On a long length carrying consistently similar traffic which has been defined for major improvement, it is important to have a comprehensive strategy to maintain an acceptable level of service and safe conditions. Ways of implementing the strategy in stages must be evolved to suit expenditure profiles. The techniques contained throughout Chapters 6 and 7 should be used when formulating the overall strategy, which, after elimination of dangerous bends, junction improvements, etc., should concentrate upon the provision of adequate Overtaking Sections. Whilst the vertical and horizontal alignments shall be coordinated in accordance with the preceding paragraphs for all newly constructed diversions and bypasses, there will frequently be little necessity for such coordination on the remaining sections which, although not conforming to formal standards, may not demonstrate any operating problems.

Staged Construction

7.47 Where a single carriageway is being considered as a first stage of an eventual dual carriageway improvement, the single carriageway shall be designed in accordance with the coordinated design aspects shown in Chapter 7. This will ensure that the impression of an essentially at-grade single carriageway road is maintained. Where it is economic to carry out some earthworks or bridgeworks for the dual carriageway in the first stage, care must be taken to ensure that the wider formation and bridges do not create the illusion of a dual carriageway. At bridges, such an illusion can be avoided by the methods described in Paragraph 7.38, and generous planting can reduce the overall impression of space.

7.48 The overriding requirements for clear Overtaking Sections in the first stage design will mean that the flowing alignment requirements for dual carriageways (as shown in Paragraph 8.7) will not be possible or desirable. However, first stage designs should be checked to ensure that the horizontal and vertical alignments are phased sufficiently to eliminate any areas where misleading visual effects in perspective might occur: for example, broken back alignments.

8. DUAL CARRIAGEWAYS AND MOTORWAYS

General Principles

8.1 All-purpose dual carriageways and motorways shall be designed to permit light vehicles to maintain the Design Speed. Subject to traffic conditions, light vehicles can overtake slower moving vehicles throughout, without conflict with opposing traffic, and drivers are free to travel at a speed controlled only by the constraints described in Chapter 1. Unlike single carriageways, therefore, there is no limitation upon the use of horizontal or vertical curves in excess of the values for one Design Speed step below Desirable Minimum values, and the coordination of design elements will mainly involve the design and optimisation of aesthetic alignments.

8.2 In the coordination of vertical and horizontal alignments, the principles contained in Paragraph 8.7 are generally desirable for all dual carriageway designs. However, for the lower categories of design, with consequently lower traffic flows, a high standard of aesthetic design may frequently not be justifiable, particularly where the dual carriageway represents an alternative to a single carriageway.

All Purpose Dual Carriageways

8.3 <u>At Grade Dual Carriageway – Category 5A (Table 4)</u>: This is the lowest category of all-purpose dual carriageway; it will normally represent an alternative layout option to single carriageway types S2 or WS2.

8.4 The vertical alignment should follow the topography closely, with the horizontal alignment phased to match. Junctions should generally be at-grade, with roundabouts at the more heavily trafficked locations, although where economically and environmentally feasible, grade separated solutions should be provided.

8.5 Major/minor junctions on dual carriageways are a source of accidents, but collecting together side roads or providing grade separation are costly alternatives that may not be economically justified. Furthermore, where the dual carriageway is being assessed as an alternative option to a single carriageway, the additional costs of higher standards of junction or alignment provision, together with the resulting higher overall earthworks and structural implications, may well cause the dual carriageway option to be so costly as to be uneconomic, in spite of its inherently superior performance in terms of link accidents and user costs. A Category 5A dual carriageway, therefore, should be designed essentially as an at-grade alternative to an at-grade single carriageway, and elements of design, such as junctions, should be enhanced only if there is economic or environmental justification for doing so. In this way, dual carriageways will frequently demonstrate superior economic performance to a single carriageway at flows well below the upper limits of single carriageway demand flows.

8.6 <u>Wide Dual Carriageway – Category 6A (Table 4)</u>: In this category, minor side roads should be stopped up, or collected together to reduce the number of gaps in the central reserve. Major intersection types, which may include roundabouts, will be determined by site conditions, traffic demand, and economic and environmental effects. The combined vertical and horizontal alignments should follow the topography as much as possible, without purposely achieving a "motorway" type of flowing alignment.

8.7 <u>Grade Separated Dual Carriageway – Categories 5B and 6B (Table 4)</u>: These are higher categories of all-purpose road, where all intersections, both major and minor (other than left in/left out minor junctions), should be grade separated. No accesses are permitted. A smooth flowing alignment is required for sustained high speeds. The following are the principles to be followed in securing a satisfactory alignment:

a) Care should be taken to ensure that embankments and cuttings do not make severe breaks in the natural skyline.

b) When negotiating a ridge in cutting or passing through a broad stretch of woodland, the road should be on a curve whenever possible so as to preserve an unbroken background.

c) Short curves and straights should not be used. Adjacent curves should be similar in length.

d) Small changes of direction should not be made, as they give the perspective of the road ahead a disjointed appearance.

e) Curves of the same or opposite sense which are visible from one another should not be connected by a short straight. It is better to introduce a flat curve between curves of the same sense, or to extend the transition curves to a common point between curves of the opposite sense.

f) Changes in horizontal and vertical alignment should be phased to coincide whenever possible. This is very important with horizontal curves sharper than 2,000m radius and vertical curves of less than 15,000m radius.

g) Flowing alignment can most readily be achieved by using large radius curves rather than straights.

h) The profile of the road over bridges must form part of the easy flowing alignment.

i) At the start of horizontal curves superelevation must not create large flat areas on which water would stand.

j) Horizontal and vertical curves should be made as generous as possible at interchanges in order to enhance sight distances.

k) Sharp horizontal curvature shall not be introduced at or near the top of a pronounced crest. This is hazardous especially at night because the driver cannot see the change in horizontal alignment.

l) The view of the road ahead should not appear distorted by sharp horizontal curvature introduced near the low point of a sag curve.

8.7A <u>High Quality Dual Carriageway – Category 5C (Table 4)</u>: This is the highest category of all-purpose road. All geometric design standards shall be in accordance with the requirements of NRA TD 9 and TD 22 for motorways. In order to obtain a smooth flowing alignment, the principles stated in Paragraphs 8.7 and 8.9 shall be followed. High Quality Dual Carriageway is only to be used where agreed with the National Roads Authority.

Motorways

8.8 The high standard of motorway design results in high speeds throughout, by complete elimination of access other than at interchanges and service areas, prohibition of usage by pedestrians and certain vehicle types, coupled with the generous flowing alignment.

8.9 The relevant alignment standards are given in Chapters 2 to 5 and the rules in Paragraph 8.7 shall be followed. Additionally:

a) Horizontal and vertical curves should be as generous as possible throughout.

b) To relieve the monotony of driving on a road with such good extensive forward visibility, long sections of the road should be aligned to give a view of some prominent feature ahead.

9. 2+1 ROADS

Introduction

9.1 There is a large gap, in terms of capacity, cost and safety, between the standard single carriageway and a dual carriageway. A number of European countries have, therefore, developed a new road type, usually referred to as a **2+1 Road**, which consists of two lanes in one direction of travel and one in the other. The two-lane section, which provides the overtaking opportunity, alternates with a one-lane section at intervals of about 1 to 2km. This is illustrated schematically in Figure 9/1. A safety barrier is required in the central reserve. See Annex D for details of recommended layouts for 2+1 junctions.

9.2 The 2+1 road is considered appropriate for use on national road schemes in rural areas. However, the approval of the National Roads Authority is required for each scheme. This chapter, accordingly, sets out design standards for such a road type, for use both on new alignments ('greenfield') or for on-line upgrading of existing roads ('retrofit'). For convenience, as well as describing alignment standards it includes details of features such as cross-section, junctions and accesses which would normally be found in other road geometry standards.

Application

9.3 Studies of capacity and economics have shown that a 2+1 road can be a more economic solution than other two-lane road options at flows of between 11,600 and 17,250 Annual Average Daily Traffic (AADT) in the design year. It is, therefore, recommended for consideration on roads with flows in that range. At the lower end of the range a 2+1 road will normally be preferable to a wide single carriageway, while at the upper end it could be considered instead of a dual carriageway. However, factors such as number of accesses, junction spacing and gradient can have a significant impact, so the feasibility of a 2+1 road should be examined on a scheme by scheme basis.

9.4 2+1 roads should not be used for urban roads or where junctions and accesses are frequent.

9.5 Where existing standard or wide single carriageway roads are exhibiting problems, it may be appropriate to improve the road by converting it to a 2+1 configuration. Such 'retrofitting' may require relatively minor alterations to the existing road, with little or no widening of the carriageway required.

9.6 Short isolated lengths of 2+1 road should not be provided. If lengths of 2+1 road within a generally single carriageway road or vice versa are unavoidable, they should be at least 5km long and preferably 10km. Junctions should be located either at the change of road type or at least 1km away.

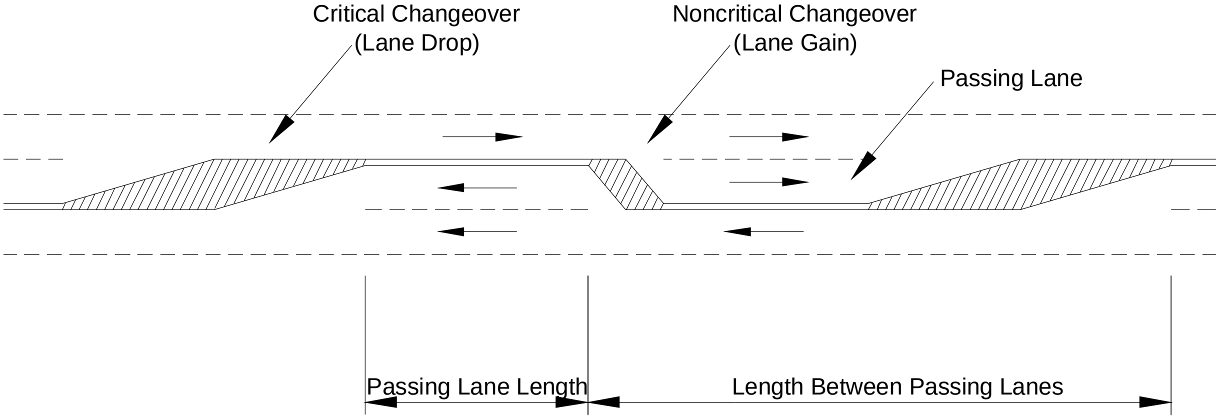


Figure 9/1: Schematic Layout of a 2+1 Road

Cross-Section

9.7 The dimensions of cross-section elements of 2+1 roads shall be as indicated in Figure 9/2 and Table 9/1. The widths of the carriageways and hard strips shall normally be in accordance with the requirements of this Interim Advice Note. Any reduction or increase in the widths of these elements would be a Departure from Standards, unless the increase results from widening on curves in accordance with Paragraphs 3.9 to 3.13.

9.8 The use of a verge or central reserve width greater than the minimum widths given in Figure 9/2 and Table 9/1 is allowed and is not considered a Relaxation or a Departure. The use of a lesser verge or central reserve width shall be recorded as a Departure. However, a width of the central reserve of less than 1.25m is prohibited.

9.9 With the exception of the particular dimensions given in Figure 9/2 and Table 9/1, all aspects of cross-section and headroom on 2+1 roads shall be in accordance with NRA TD 27. This includes clearances at structures.

9.10 2+1 roads on new alignments shall be designed to the widths shown in Figure 9/2a. Where an existing road is to be revised to form a 2+1 road, every effort should be made to provide a paved width of 14m. However, at confined locations on existing roads the paved width may be reduced to a minimum of 13m as shown in Figure 9/2b.

9.11 In all cases, the central reserve shall be paved and a safety barrier shall be provided, unless otherwise approved by NRA.

9.12 At restricted locations, for example on a long viaduct or at a difficult pinch point, the cross-section may be reduced to omit the passing lane. The resulting cross-section should be as shown in Figure 9/3a, to form a '1+1 road', which is the same as a 2+1 road except there are two one-lane sections side by side. This layout will avoid the confusion that could occur if the cross-section were to be changed to that of a single carriageway road and back again (see Paragraph 9.40). On 'retrofit' schemes, the paved width may be reduced to a minimum of 10.25m as shown in Figure 9/3b.

2+1 Roads with Overlapping Passing Lanes

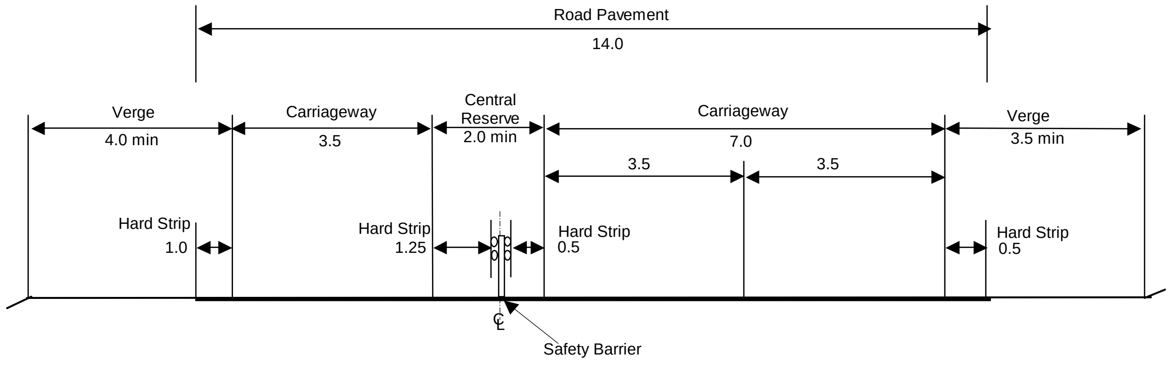
9.13 In some instances, it may be appropriate to provide two overlapping two-lane sections, thus forming a '2+1 Road with Overlapping Passing Lanes'. This is most likely to occur on long hills where the uphill two-lane section functions as a climbing lane (see Paragraphs 5.48 to 5.53). The cross-section at such locations should be as shown in Figure 9/4a. At such locations, the minimum width of central reserve is 2.6m, the same as for a standard dual carriageway (see NRA TD 27). For restricted locations on retrofit schemes Fig 9/4b shall apply. Note the provision of separate Pedestrian/cyclist facility on Fig. 9/4b.

Alignment

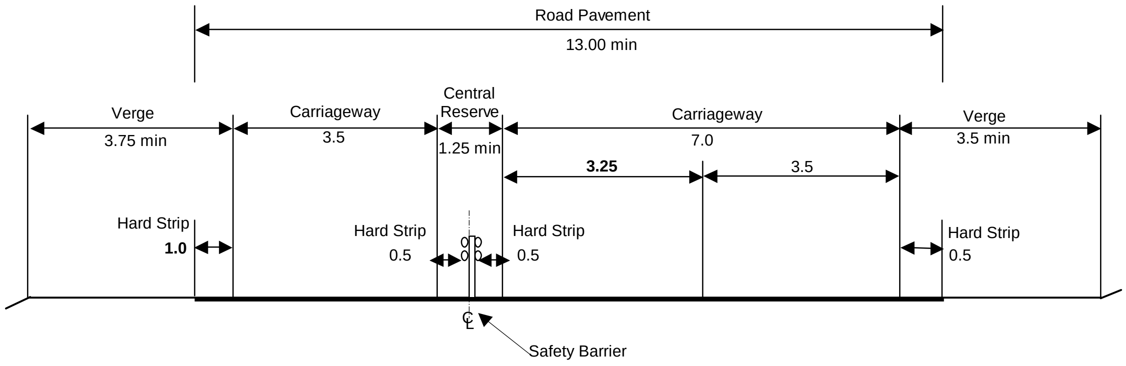
9.14 2+1 roads should be designed with the objectives of safety and uncongested flow in mind. This will require not only appropriate design of the horizontal and vertical alignments but also careful attention to the arrangement of the two-lane sections and the location of junctions.

9.15 Standards for the alignment of 2+1 roads are set out in Chapters 1 to 4 of this Interim Advice Note. The horizontal and vertical alignments should be coordinated to produce a smooth and flowing layout: the principles given in Paragraph 8.7 should be followed wherever practicable. The restrictions on the use of Band C curves and the requirements for Full Overtaking Sight Distance described in Chapter 7 do not apply to 2+1 roads.

9.16 On new alignments, the crown for crossfall or camber shall be located within the central reserve, preferably at the centre, where superelevation does not apply. However, when an existing road is converted to a 2+1 road, it is acceptable for the crown to be within a traffic lane.



a) Preferred Cross-Section



b) Reduced Cross-Section (See Paragraph 9.10)

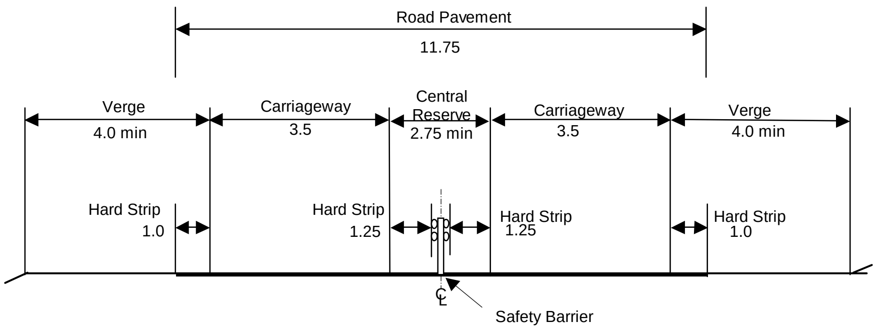
Note: All dimensions are in metres.

Figure 9/2: Cross-Section Widths for 2+1 Roads

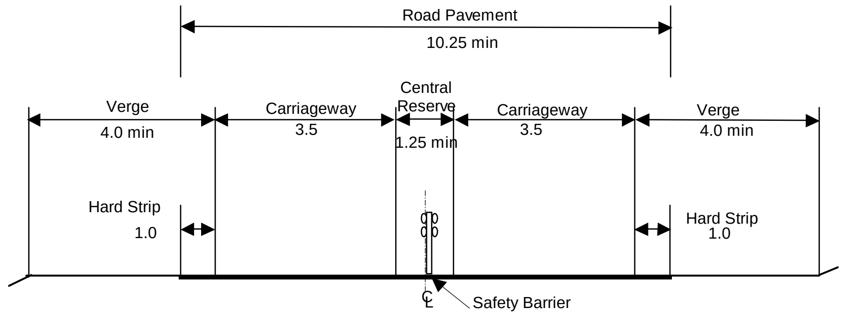
Table 9/1: Dimensions of Cross-Section Elements for 2+1 Roads

	Nearside			**Offside**				**Nearside**	
	Verge	**Hard Strip**	**Carriageway (Single Lane)**	**Hard Strip**	**Central Reserve**	**Hard Strip**	**Carriageway (Two-Lane)**	**Hard Strip**	**Verge**
(see Notes)	1, 3	2	2	2	1, 3	2	2	2	1, 3
MAINLINE									
2+1 Road Preferred Cross-Section	4.0 min	1.0	3.5	1.25	2.0 min	0.5	7.0	0.5	3.5 min
2+1 Road Reduced Cross-Section (see Para 9.10)	3.75 min	1.0	3.5	0.5	1.25 min	0.5	6.75	0.5	3.5 min

Notes:
1. Verge and central reserve dimensions are desirable values: any reduction will require a Departure.
2. Carriageway and hard strip dimensions are fixed values: any alternative is a Departure.
3. Where a hard strip is present, the corresponding verge or central reserve dimension includes the hard strip.
4. All dimensions are in metres.



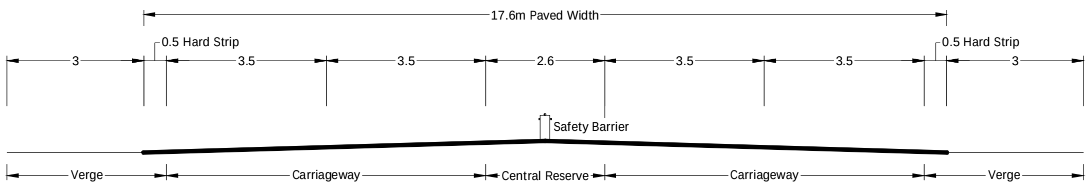
a) Preferred Cross-Section



Note: All dimensions are in metres.

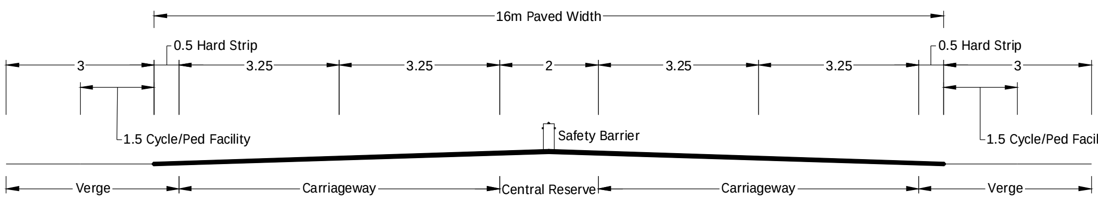
b) Reduced Cross-Section (see Paragraph 9.12)

Figure 9/3: Cross-Section Widths for '1+1 Roads' in Restricted Locations



Note: All Dimensions are in Metres

Figure 9/4 (a): Cross-Section Widths for '2+1 Roads' with Overlapping Passing Lanes



Note: All Dimensions are in Metres

Figure 9/4 (b): Cross-Section Widths for '2+1 Roads' with Overlapping Passing Lanes at Restricted Locations with shared pedestrian/cycleway facilities.

Passing Lane Length

9.17 In order for a 2+1 road to be effective, the traffic in both directions needs to be given opportunities to overtake. Thus, the side of the carriageway with two lanes (the passing lane) needs to change over at intervals. The length of an individual two-lane section is a compromise between allowing a length long enough to enable a platoon of traffic to overtake a slower vehicle on the two-lane side and short enough to avoid causing delay and frustration to traffic on the one-lane side. Lengths will also be determined by the road geometry, location of junctions, etc. Passing lanes should have a full width length of between 1,000m and 2,000m. A full width length of between 800m and 1,000m would be a Relaxation, while a full width length less than 800m would be a Departure from Standard.

9.18 Long single lane lengths should be avoided: a length between full width passing lanes in the range from 2,500m to 3,000m would be a Relaxation, while a length greater than 3,000m would require a Departure from Standard.

Changeovers

9.19 Locations where the passing lane changes from use by one direction of travel to use by the other are '**changeovers**'. There are two principal types. A '**critical**' changeover is one immediately downstream of a lane drop (see Figure 9/1). It is called critical because vehicles in the middle lane are heading towards one another, so a substantial buffer is needed. A '**non-critical**' changeover is one immediately upstream of a lane addition. This is not so critical as vehicles in the middle lane are heading away from one another.

9.20 Changeovers should be sited at junctions where practicable (see Paragraph 9.21). However, where changeovers occur away from junctions they shall be in accordance with the layouts shown in Figures 9/5 and 9/6. Critical changeovers shall not be permitted where the curve radius is Band D or below, as per Figure 24 of TD9.

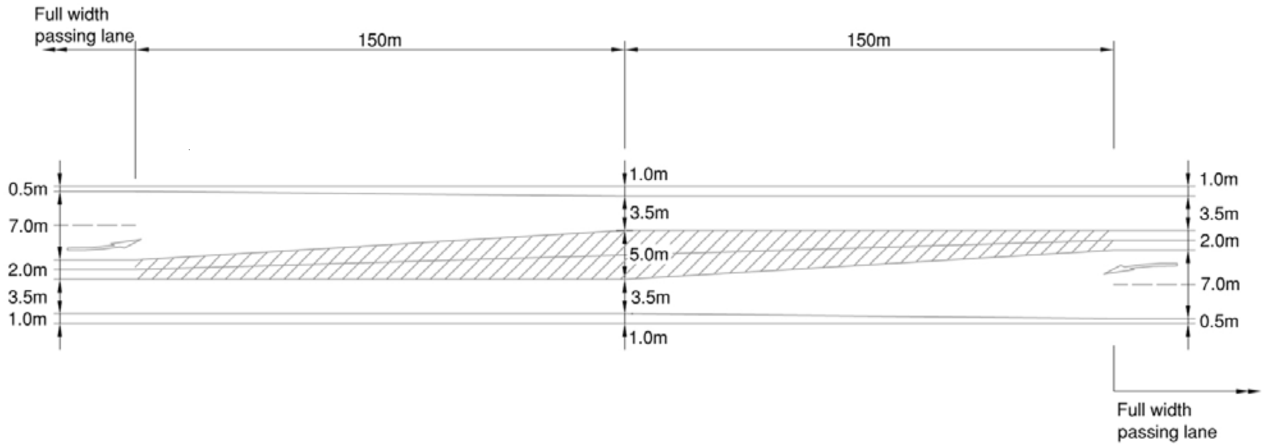


Figure 9/5: Dimensions of Critical Changeover (Lane Drop)

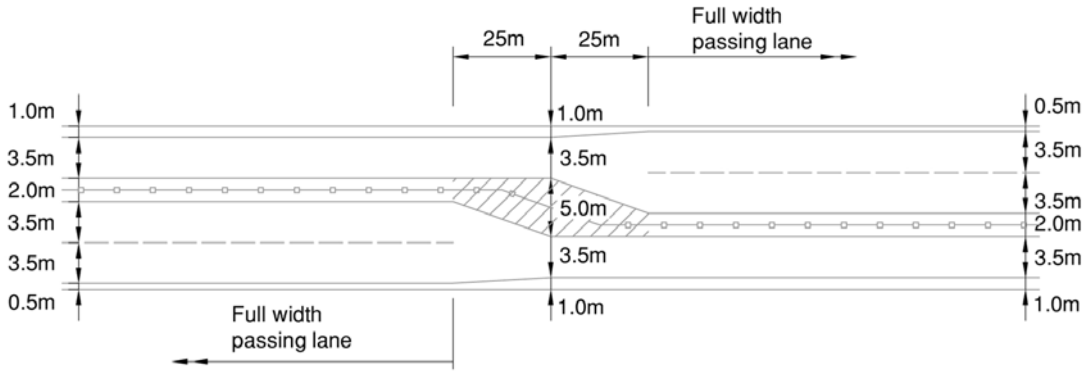


Figure 9/6: Dimensions of Non-critical Changeover (Lane Gain)

Junction Strategy

9.21 2+1 roads should be designed so as to minimise the number of junctions and to provide drivers with straightforward junction layouts. Wherever practicable, junctions should be located at changeovers. The following junction types are permitted for use on 2+1 roads, as described below:

- Left-in/left-out;
- Right-turn priority junction with ghost islands;
- U-turn facility with right turn;
- Staggered junction with ghost islands;
- Roundabout; and
- Compact grade separation.

9.22 The aim should be to provide drivers with layouts that have consistent standards and are not confusing. On lengths of 2+1 road, sequences of junctions should not therefore involve many different layout types. For example, a length of route containing roundabouts, priority junctions and grade separation would inevitably create confusion and uncertainty for drivers and cause accidents on that account. The safest road schemes are usually the most straightforward ones that contain no surprises for the driver.

Typical Junction Layouts

9.23 Junction layouts should be in accordance with the geometric standards of TD 42, together with the following additional requirements. Layouts should be derived by assembling the relevant components for each part of the junction, as described in TD 42. At some locations on 'retrofit' schemes, junction layouts in accordance with the following paragraphs may not be practicable. In such locations approval for relevant Departures from Standards will need to be sought from the NRA.

Left-in/Left-out Junctions

9.24 Junctions occurring within or opposite passing lane lengths (on either the two-lane or one-lane side) should be avoided where practicable: it is safer to concentrate turning traffic at ghost island junctions at changeovers. Where junctions are provided on either the one-lane or two-lane side, they shall be restricted to left-in and left-out turning movements only, with no crossing of the central reserve. A typical layout is shown in Figure 9/7. This junction type is only suitable for lightly trafficked minor roads or private accesses. Such junctions shall not be provided within 100m of a critical changeover. Where junctions of this type are required on opposite sides of the road, the side roads shall be staggered by a nominal 10m to avoid the appearance of a through road.

Right-turn Priority Junctions

9.25 Priority junctions permitting right turns shall be sited at changeovers. Therefore the passing lanes are terminated either side of the junction, leaving a ghost island at the junction, with one 3.5m lane in each direction. Typical layouts are illustrated in Figures 9/8 and 9/9 and relevant dimensions are tabulated in Table 9/2. Hard strip widths shall be adjusted to 0.75m through the junction in order to provide a right turn lane width of 3.5m and a ghost island width of 2.0m. Apart from the dimensions shown in the figures, the layout should be in accordance with the geometric standards of TD 42.

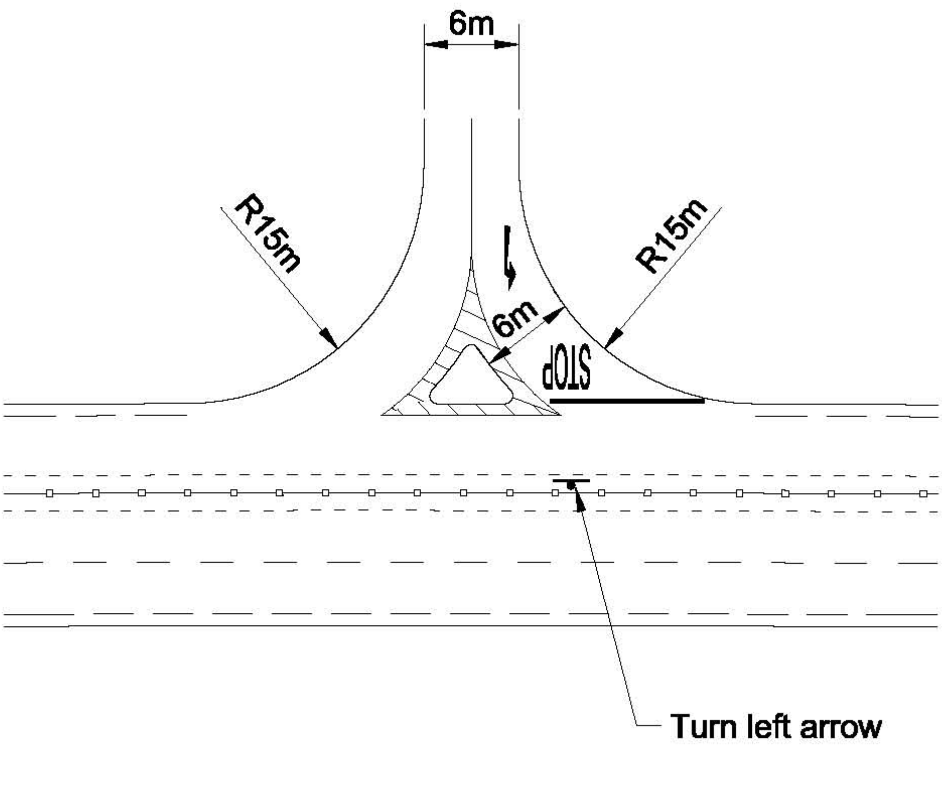


Figure 9/7: Left In/Left Out Junction Layout

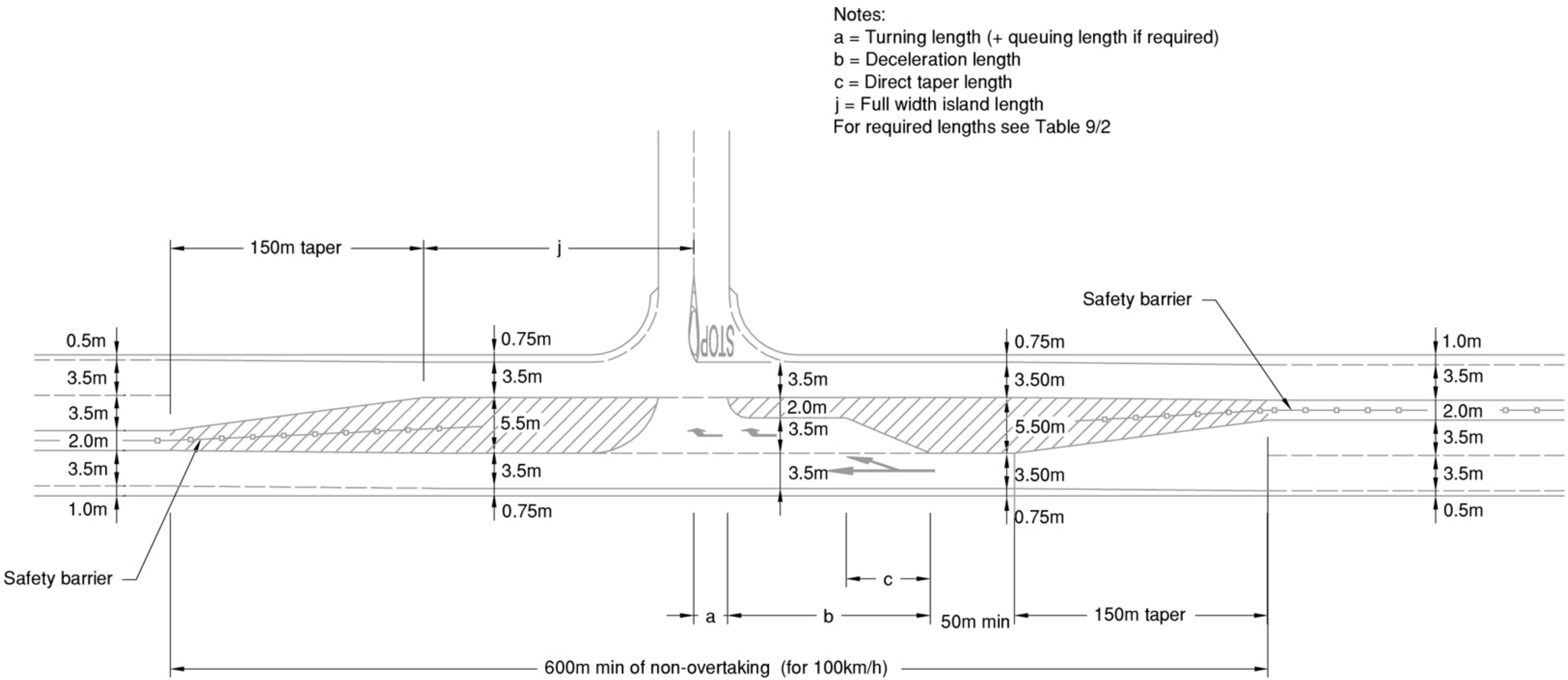


Figure 9/8: Junction Layout at Critical Changeover

Table 9/2: Required Dimensions at Junctions

Design Speed of 2+1 Road (km/h)		100 km/h	85 km/h	70 km/h
a	Turning Length (and Queuing Length if required: see TD 42)	10 m	10 m	10 m
b	Deceleration Length	80 m	55 m	40 m
c	Direct Taper Length	25 m	15 m	15 m
j	Full Width Island Length	160 m	120 m	90 m

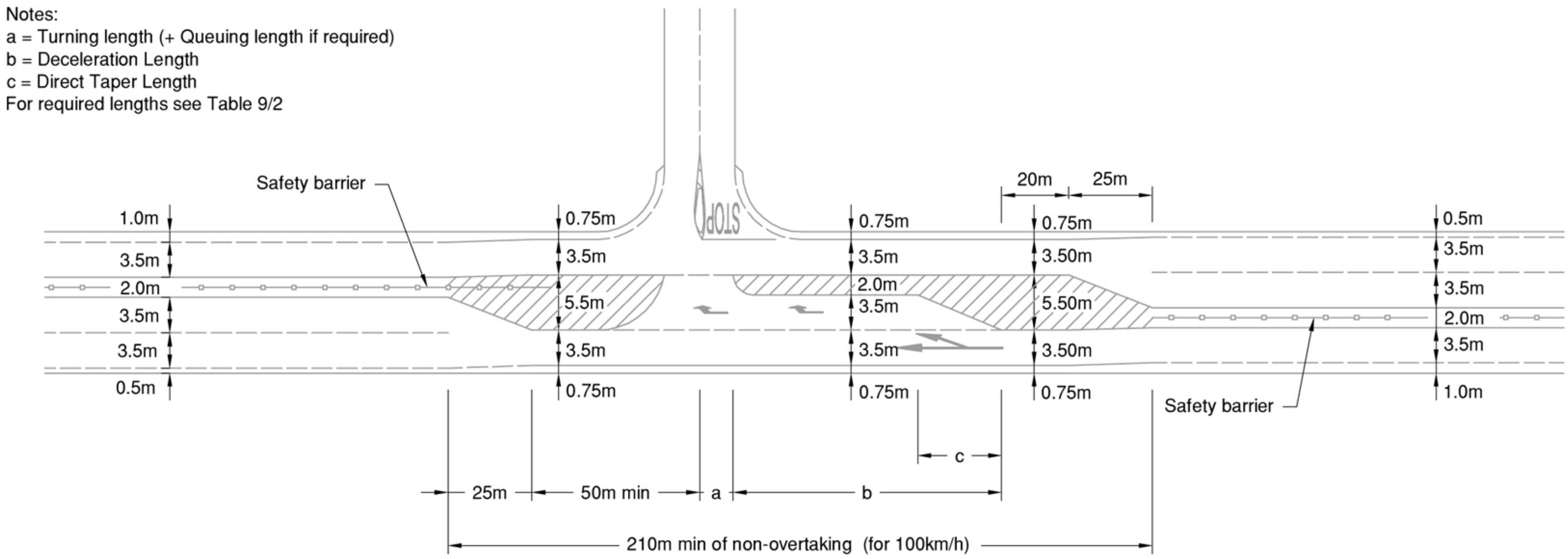


Figure 9/9: Junction Layout at Non-critical Changeover

U-Turn Facilities

9.26 On some lengths of 2+1 road where there are several accesses or junctions permitting left-in/left-out only, there may be a need to allow for vehicles wishing to make U-turns in order to minimise inconvenience to landowners and other users of the minor junctions. Roundabouts provide a safe way of allowing U-turns at junctions, but it would not normally be appropriate to construct a roundabout just for U-turns.

9.27 U-turn facilities can be provided at right turn priority junctions by providing a turning loop in the minor road, as illustrated in Figure 9/10. Such a layout allows U-turns in either direction.

9.28 Where it is necessary to provide a U-turn facility away from a right turn priority junction, a mainline U-turn loop can be provided at a minor road or a private access, as illustrated in Figures 9/11 and 9/12. The preferred location for such layouts is at a changeover. Where a U-turn loop is provided at a passing lane length, the loop shall not be provided on the two-lane side, so as to avoid the need for turning traffic to cross two through lanes. It will be noted that these U-turn facilities permit right turns into the side road or access but not out. They also allow U-turns in one direction only. Similar U-turn loops could be provided where there is no minor road or private access, but it is better to combine a loop with a side road or access.

9.29 Careful signing will be required on the approaches to all U-turn facilities (except roundabouts) to alert drivers to the U-turn loop.

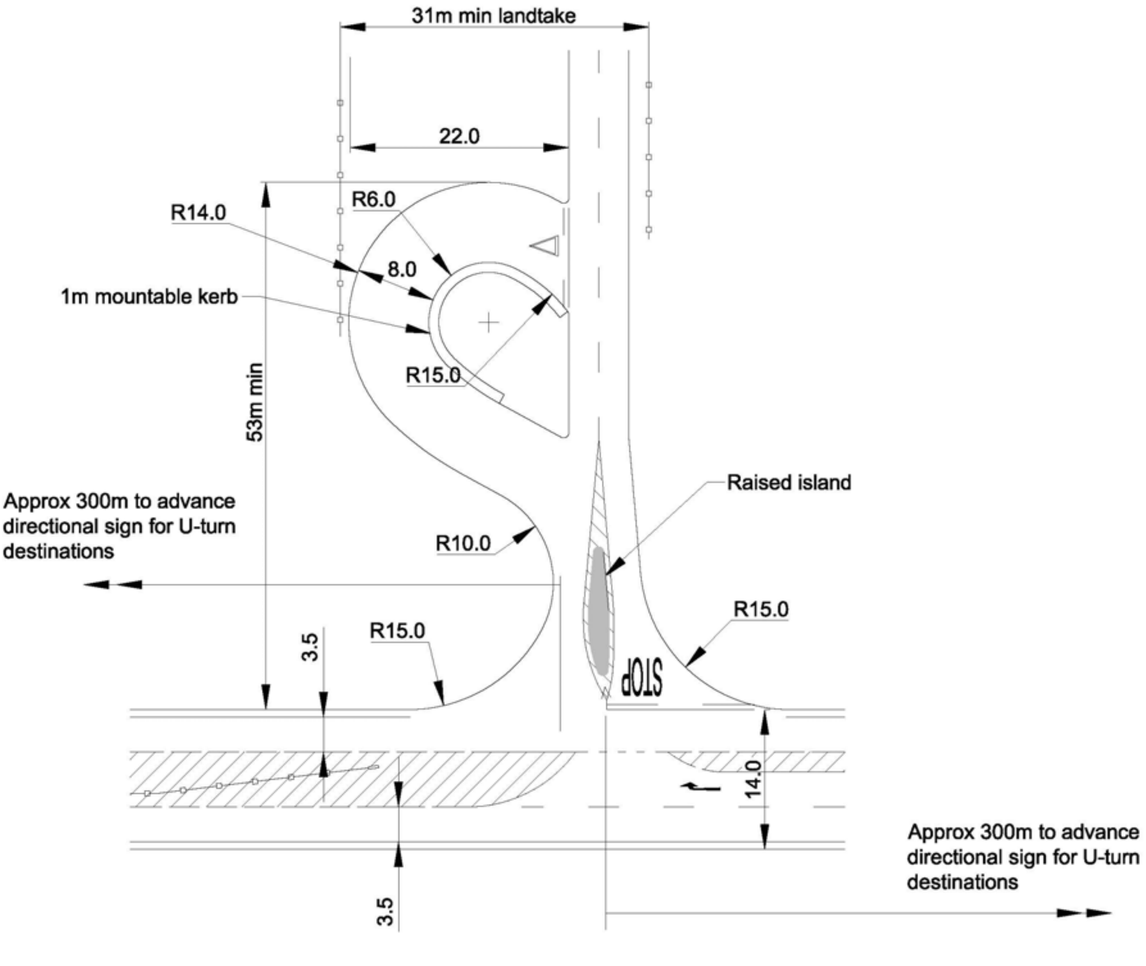


Figure 9/10: U-Turn Loop in Minor Road

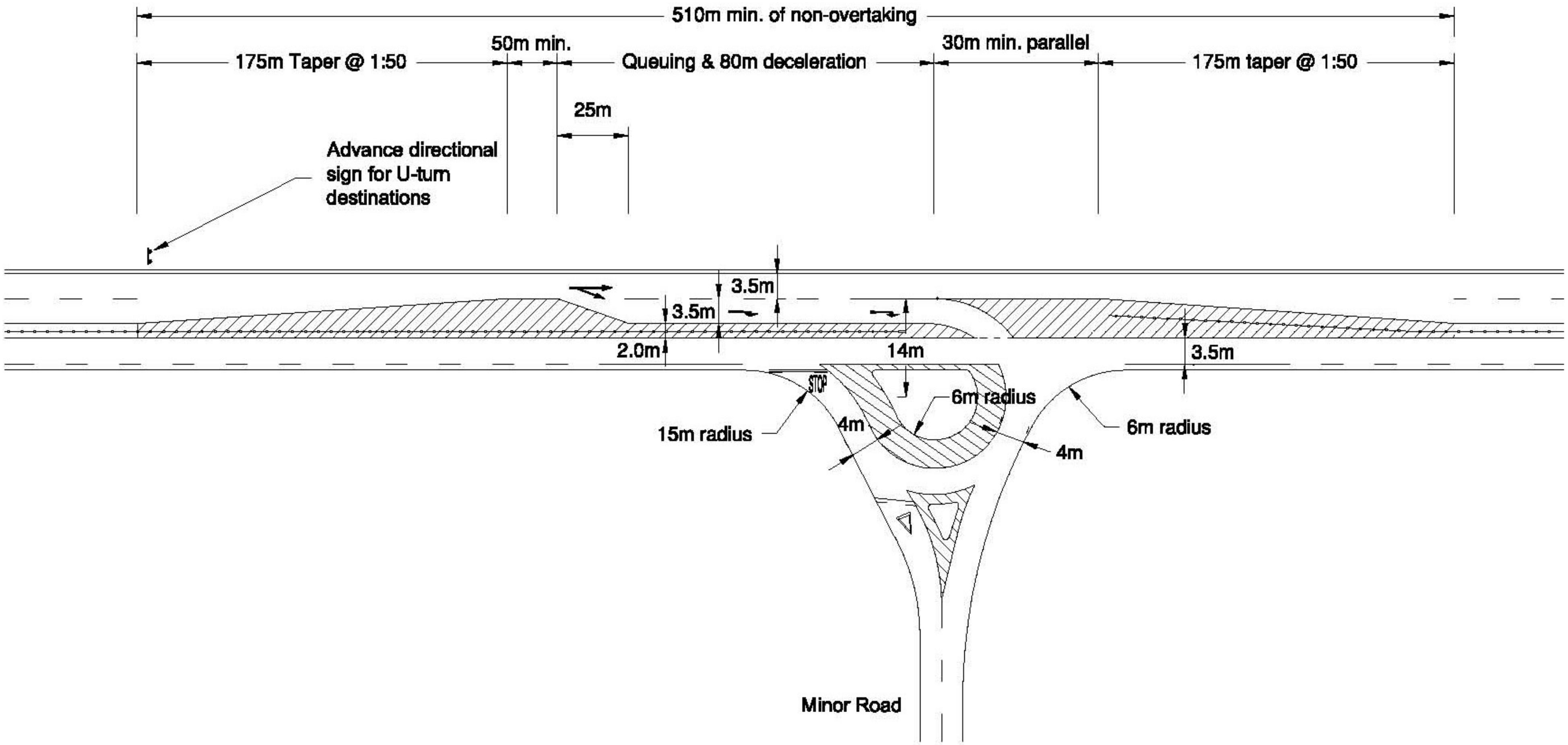


Figure 9/11: Mainline Turning Loop at Minor Road

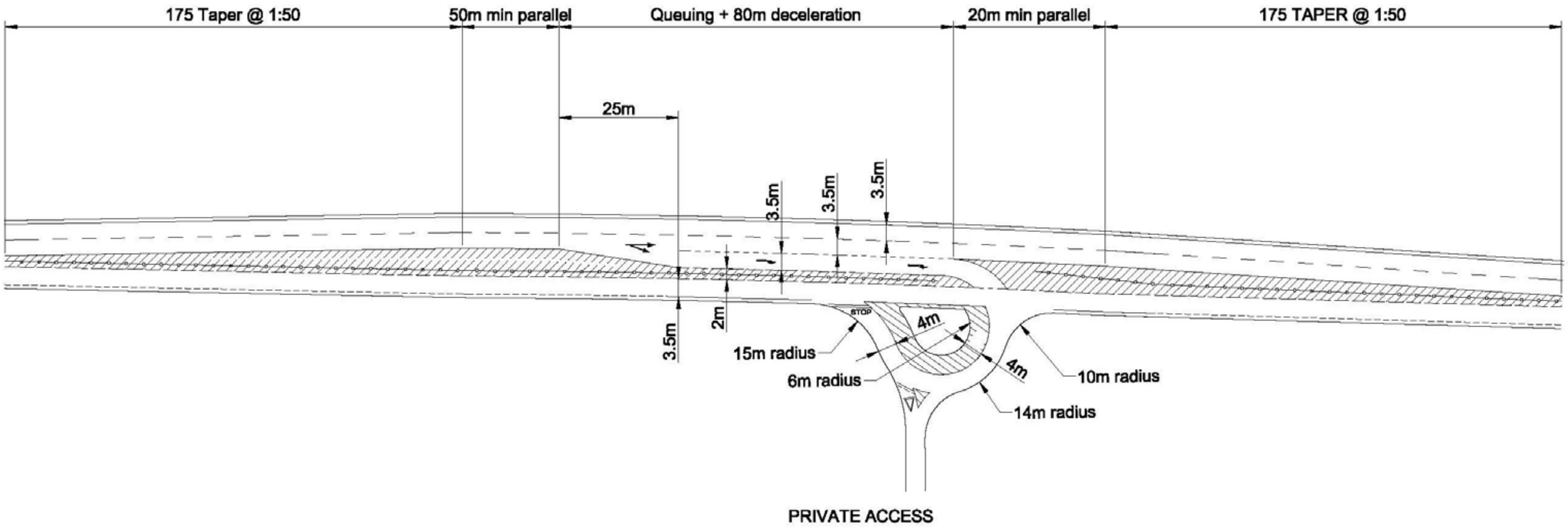


Figure 9/12: Mainline Turning Loop at Private Access

Staggered Junctions

9.30 Crossroads are not permitted on 2+1 roads. Staggered junctions may be provided only at changeovers – not at passing lanes. Right/left staggers (where minor road traffic crossing the major road first turns right, proceeds along the major road and then turns left) are preferred to left/right staggers because traffic turning between the minor roads is less likely to have to wait in the centre of the major road.

9.31 The stagger distance of a junction is the distance along the major road between the centrelines of the two minor roads. The minimum stagger distance for a right/left stagger shall be 50m. For a left/right stagger, it shall be 100m for a 100km/h design speed, 75m for an 85km/h design speed and 60m for a 70km/h design speed. These left/right stagger distances are based on the sum of the two deceleration lengths lying side by side plus the turning lengths (and queuing lengths if appropriate) at each end.

9.32 The layouts of staggered junctions should be based on the layout for right-turn priority junctions as shown in Figures 9/8 and 9/9.

Roundabouts

9.33 Roundabouts will be appropriate for major junctions on many 2+1 roads. They should be designed in accordance with the requirements of TD 16. Two-lane sections may start directly at the exit from the roundabout. Similarly, a two-lane section may terminate at a roundabout with the overtaking lane becoming the right-hand entry lane into the roundabout.

Compact Grade Separated Junctions

9.34 On 2+1 roads with relatively high traffic flows, it may be appropriate to provide compact grade separated junctions. Such junctions shall be designed in accordance with TD 40. On a length of 2+1 road with grade separated junctions there shall be no breaks in the central reserve. All major junctions shall be grade separated, while minor ones shall be left-in/left-out only. U-turns will only be possible at the grade separated junctions. Accesses should be severely limited or avoided altogether. Where practicable, layouts should be designed so that merging occurs on two-lane sections, thereby avoiding the problems of merging into a single lane. This can be achieved if the junction is at a non-critical changeover.

Vehicular Accesses

9.35 Direct vehicular access onto 2+1 roads from private property or developments should be avoided as far as practicable. Where accesses are to be provided, the requirements of TD 41 shall be adhered to. Accesses shall normally be permitted for left-in/left-out turns only, as illustrated in Figure 9/13. However, where it is necessary to combine a u-turn facility with a private access, as shown in Figure 9/12, the right turn in movement is also permitted. Left-in left-out accesses of the type shown in Figure 9/13a shall not be permitted within the length of a critical changeover nor within 150m on the approach to a critical changeover.

9.35a All entrances should be checked to ensure that they can cater for the swept path of appropriate vehicles.

9.35b Temporary crossings for farm access shall only be allowed with the express permission of the NRA. These crossing points shall only be made available under the strict supervision of the Local Authority and shall be used only by exception. Five days advance notice shall be given to the Local Authority Area Engineer prior to opening of the crossing. For details of crossing layout see Fig 9/13b.

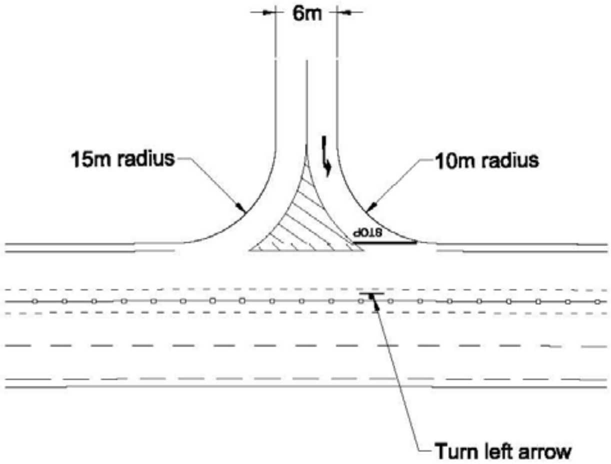


Figure 9/13a: Vehicular Access

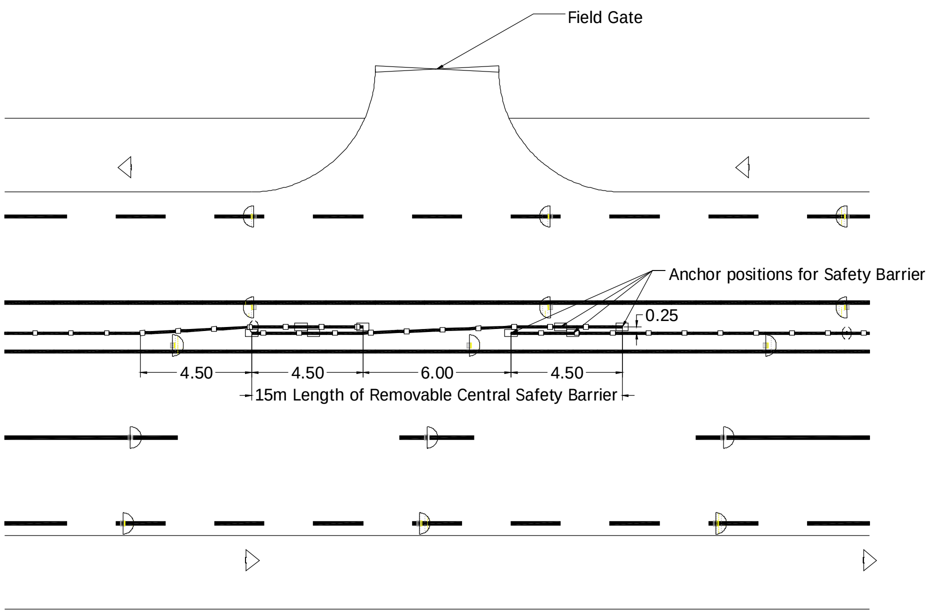


Figure 9/13b: Field Access

Visibility at Junctions

9.36 Visibility at junctions shall be in accordance with the requirements of TD 42, except as described below. At junctions restricted to left in/left out, a single visibility splay to the right will suffice. At junctions where a crossing of the central reserve is permitted, the visibility splay to the left may be taken to the central reserve edge of the far carriageway on the major road. The central reserve safety barrier will need to be terminated either side of the junction to avoid encroaching into the visibility splays; however, the gap in the barrier should be kept to a minimum. See Annex D, recommended layouts for 2+1 junctions.

Merging and Diverging Tapers

9.37 Merging and Diverging tapers shall not be used on 2+1 roads.

Pedestrian and Cyclist Requirements.

9.38 Where a pedestrian / cyclist facility is required it shall be provided in accordance with Fig. 9/4b.

9.39 Where a crossing demand for pedestrians exists it shall only be provided at 1 plus 1 sections and in accordance with Fig. 9/13c.

Changes in Carriageway Type

9.40 Where it is appropriate to change from a 2+1 road to a single or dual carriageway careful consideration should be given to the use of a roundabout as a terminal junction. A roundabout slows all traffic and helps to indicate the change of cross section.

9.41 Where there is a change between a 2+1 road and a single carriageway, other than at a roundabout, the preferred arrangement is for traffic leaving the 2+1 road to be on a one lane length and traffic entering the 2+1 road to join a two-lane length. The start of the 2+1 road shall be preceded by a taper of 1/50, as shown in Figure 9/14. If traffic leaving the 2+1 road is on a two-lane length, it will be necessary to reduce that side of the carriageway to a single lane section in a manner similar to a critical changeover.

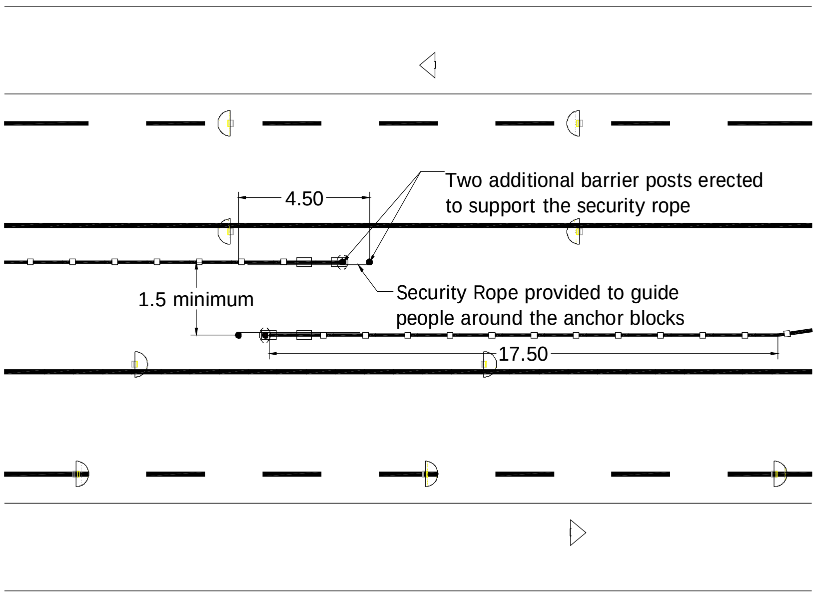


Figure 9/13c Pedestrian Crossing Facilities at 1+1

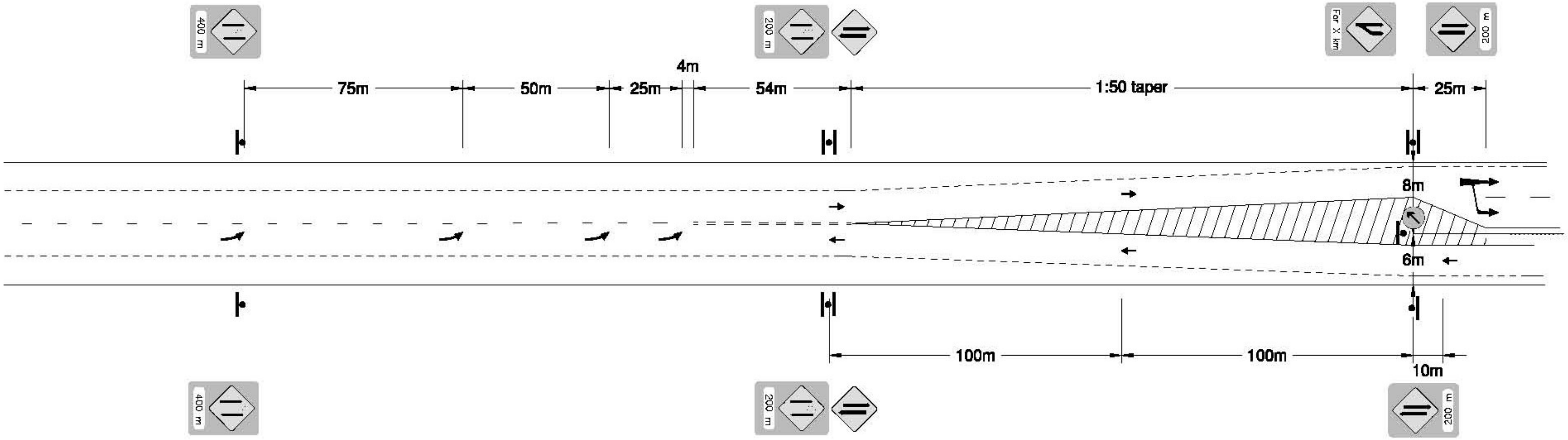


Figure 9/14: Change Between Standard Single Carriageway and 2+1 Road

Passing Bays

9.42 To allow for breakdowns and very slow vehicles, passing bays should be provided near the mid length of each one-lane length on a 2+1 road. The layout of the passing bay should be in accordance with Figure 9/15. Wherever practicable, passing bays should not be sited on the inside of bends or near junctions or signing. This facility can also function as a bus bay.

Safety Barriers

9.43 2+1 roads shall be provided with a safety barrier in the central reserve. The barrier shall have a Containment Level of N2 and an Impact Severity Level of A, and a Working Width of W6 (2.1m) or less, in accordance with EN 1317-2 (see NRA TD 19). The barrier width shall not exceed 0.25 m in order to fit within the central reserve provided under the standard and so as not to encroach onto the hard strips. The central reserve safety barrier shall be in accordance with NRA TD 19 except that the Working Width shall be such that under design impact conditions the barrier will be permitted to deflect into the opposing traffic lane.

9.44 To allow for emergencies and for maintenance after an impact, the barrier assembly, as tested in accordance with EN 1317 part 2, shall be capable of being dismantled and reassembled by two or three people without hand power tools or machinery. If a contraflow situation is required (after an accident or breakdown for example) the system shall be capable of allowing the vertical or support members to be removed and the longitudinal members to be lowered to ground level for a minimum distance of 17 m at any or multiple points along its length to let vehicles pass over uninhibited and safely. The system shall be such that it can be reassembled manually without the use of hand power tools or machinery back to its original position and assembly, as tested in accordance with EN 1317 part 2.

9.45 Ramped terminals shall be provided at the ends of runs of central reserve safety barrier. The terminals shall be such that all parts above the surface of the road lie between the hard strips. At junctions and other locations where vehicles cross the central reserve, the safety barrier terminals shall be located as close to the junction as practicable, subject to the visibility requirements for the turning traffic. It’s imperative that there is a stagger between the ends of the barrier. Refer to Annex D for details of safety barrier positioning at right turn lane junctions.

9.46 Relaxations of up to two Design Speed steps below the Desirable Minimum Stopping Sight Distance are permitted for visibility to the low object at the central reserve safety barrier, provided Desirable Minimum Stopping Sight Distance is obtained to a 1.05m high object (see Paragraphs 2.7A, 1.25 and 1.28).

9.47 Safety barriers in the verges of 2+1 roads shall be in accordance with the requirements of NRA TD 19.

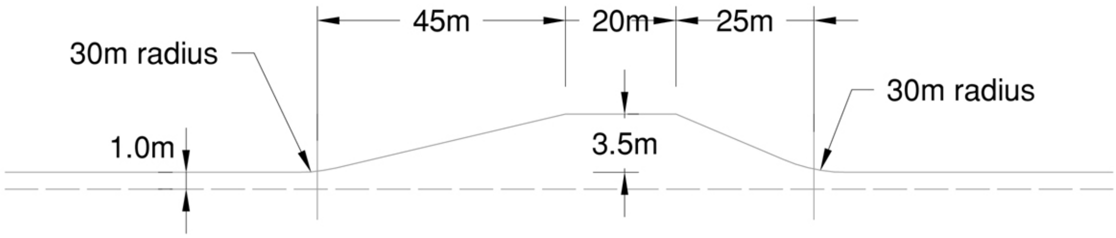


Figure 9/15: Passing Bay

Traffic Signs and Road Markings

9.48 The start and end of each passing lane on a 2+1 road shall be signed as indicated in Figure 9/16. Where the changeover occurs at a junction, the traffic signs indicating the start and end of the passing lanes shall be positioned so as not to conflict with the signs for the junction.

9.49 Signs at junctions on 2+1 roads shall be in accordance with the Traffic Signs Manual, based on the requirements for signing at junctions on dual carriageways. Stop signs and road markings shall be provided on the minor roads.

9.50 Where a U-turn facility is to be provided, directional signs shall be used to indicate the paths to be followed by all turning traffic, as shown in the example in Figure 9/17.

9.51 Where a length of 2+1 road joins onto a length of single carriageway, the signs for 'road divides' and 'dual carriageway ends' shall be provided in accordance with the Traffic Signs Manual to indicate the start and end of a central reserve (see Figure 9/14).

9.52 Continuous white lines, in the form of raised rib road markings, shall be provided to mark the edges of the central reserve. Hatching is not required in the central reserve except at changeovers and junctions.

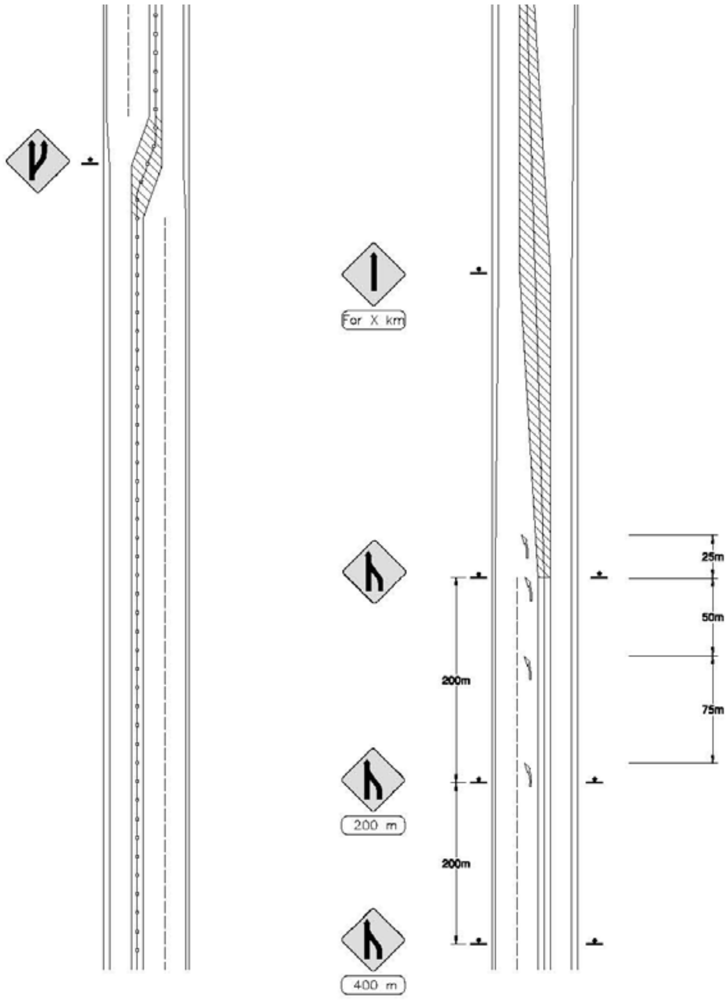
9.53 The locations of reflectorised discs on the safety barrier shall be as shown in Annex D.

Monitoring of 2 + 1 Roads

9.54 Monitoring of 2+1 schemes shall be in accordance with Annex B.

Maintenance of 2 +1 Roads

9.55 It is vitally important that the schemes are well maintained, inspected at regular intervals and that any damage to the safety barrier in repaired within the stated timeframes. See Annex C for details of a typical maintenance regime.



a) Start of Passing Lane **b) End of Passing Lane**

Notes: 1. Figure shows signs for one direction of travel only.
2. Signs at the end of the passing lane to be erected on both sides of the road.

Figure 9/16: Signs at Start and End of Passing Lane

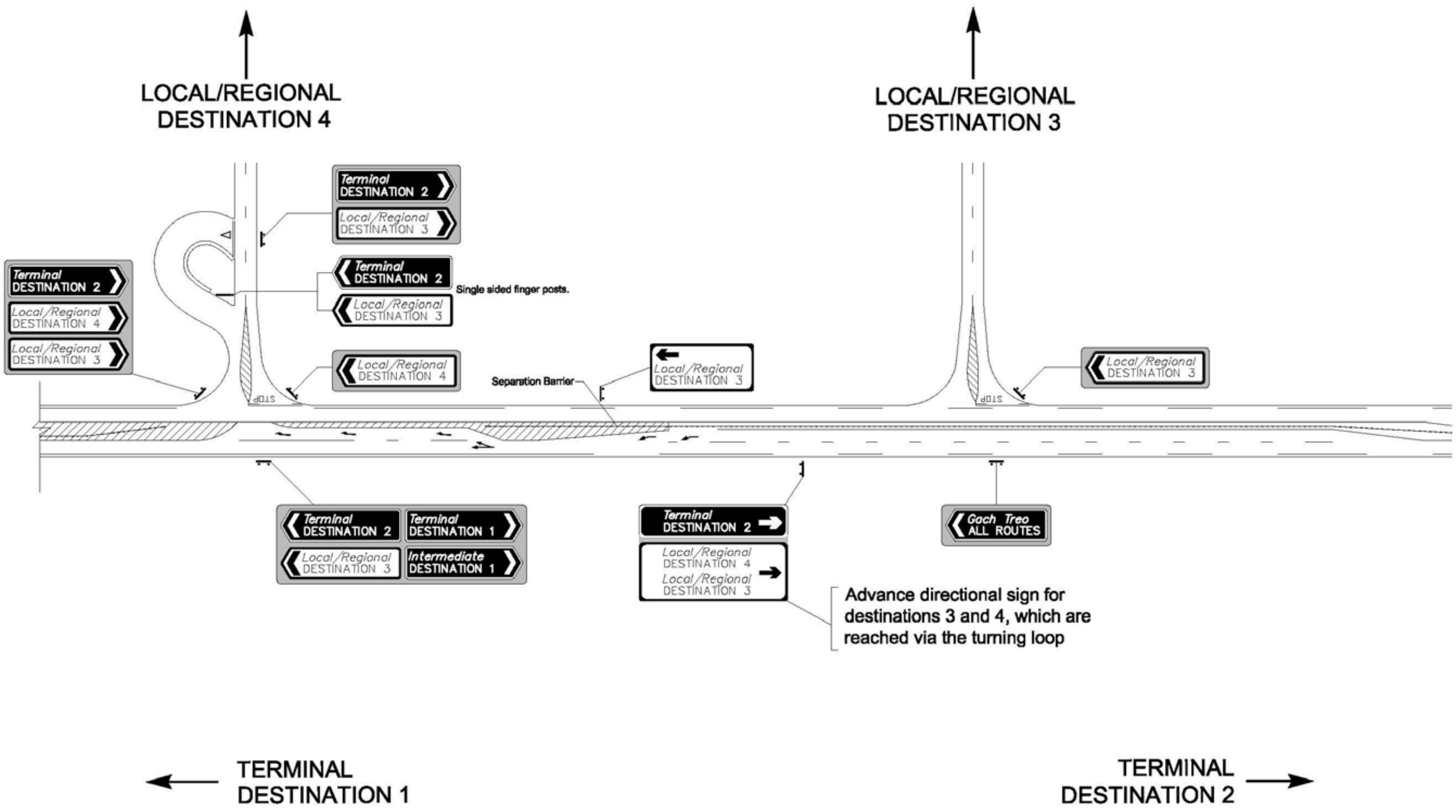


Figure 9/17: Directional Signs at U-Turns

10. IMPROVEMENTS TO EXISTING NATIONAL ROUTES

Minor Improvements on Existing National Roads

10.1 Many existing roads contain sub-standard geometric design features. Every effort should be made, therefore, to improve the alignment whenever works are undertaken: the improvements should conform to the standards of NRA TD 9 where practicable. Where the Desirable Minimum standards cannot be achieved, it may be necessary to consider Relaxations or Departures from Standard. The procedures set out in Paragraphs 1.15 to 1.31 above should be used for considering options, recording Relaxations and applying for Departures. These procedures should be applied for all remaining or proposed features which are less than Desirable Minimum. However, where a short length of existing road (up to 200m long) is to be improved as a stand-alone scheme, or where the scheme is for the introduction of traffic safety measures, the design organisation may seek the agreement of the National Roads Authority for any Departures in the proposed layout to be considered as Relaxations. If this agreement is given, the design organisation may record Departures as Relaxations in accordance with Paragraph 1.17, without the need to apply for individual approvals.

Pavement Overlays/ Widening on National Routes

10.2 In the case of pavement overlay schemes where the average width of individual lanes (including running strips) remains unchanged, trim back overgrown verge and restore/reconstruct the pavement to its original width.

10.3 Where pavement overlay schemes are proposed that will result in road widening the 85%ile speed of the road shall be determined (by site measurement) to establish the design speed of the road.

If the widening scheme results in changing the operating characteristics of the road, accommodation works shall be incorporated in the pavement design proposals that will ensure that that access to the new road via minor road junctions (and from domestic and agricultural entrances) is no less favourable post works than pre works.

If treatments (i) to (iv) outlined below are not practicable or cannot achieve no less favourable road conditions at reasonable cost, the widening scheme should not be implemented at these locations.

The following are examples of possible treatments that could be adopted as part of a proposed pavement widening scheme:

(i) Where the desirable minimum sight distance is less than that required for the 85%ile speed of this section of road, agricultural entrances shall be recessed to provide refuge off the line of the road as per standard detail NRA RCD 2700/101.

(ii) The sight distance provided at domestic entrances shall be at least one step below the Desirable Minimum for the 85%ile speed of this section of road. This shall be provided to the Right Hand side of the domestic entrance measured from a point 2.4m from the near carriageway running edge.

Hatching of the hard shoulder or running strip, in advance of domestic entrances, should be considered and utilised where practicable (so as to provide some degree of ‘protection’ to those accessing the road).

(iii) In the case of junctions with regional or national roads, the minimum sight distance shall be the Desirable Minimum for the 85%ile speed of this section of road.

(iv) In the case of local road junctions, the minimum sight distance shall be at least one step below the Desirable Minimum for the 85%ile speed of this section of road, and shall be provided at the access position in both directions.

(v) Where the geometrics of the existing road are such that the minimum sight distances given in paragraphs (i) to (iii) cannot be provided at reasonable cost, consideration should be given to reducing the maximum speed on the road to 80km/h along the length under consideration (Note the Special Speed limit procedure requirement). The requirements and associated minimum sight distances outlined in (ii) and (iii) above should relate to the reduced operational speed of the section of road.

NB: A Departure from Standard will be required where a reduction in the maximum speed limit of the road is proposed in order to achieve a standard of one step below the Desirable Minimum.

11. REFERENCES

BS 6100 : Subsection 2.4.1, Glossary of Building and Civil Engineering Terms: Highway Engineering. British Standards Institution.

NRA Design Manual for Roads and Bridges (NRA DMRB):

NRA TA 43 (NRA DMRB 6.1.1A) – Guidance on Road Link Design.

NRA TA 69 (NRA DMRB 6.3.3) – The Location and Layout of Lay-bys.

NRA TD 19 (NRA DMRB 2.2.8A) – Safety Barriers.

TD 22 (DMRB 6.2.1) – Layout of Grade Separated Junctions.

NRA TD 27 (NRA DMRB 6.1.2) – Cross-Sections and Headroom.

TD 40 (DMRB 6.2.5) – Layout of Compact Grade Separated Junctions.

TD 41 (DMRB 6.2.7) – Vehicular Access to All-Purpose Roads.

TD 42 (DMRB 6.2.6) – Geometric Design of Major/Minor Priority Junctions.

Guidelines on Traffic Calming for Towns and Villages on National Routes. National Roads Authority.

National Roads Project Management Guidelines. National Roads Authority.

Road Traffic (Signs) Regulations.

Traffic Signs Manual. Department of the Environment and Local Government.

12. ENQUIRIES

12.1 All technical enquiries or comments on this Interim Advice Note should be sent in writing to:

E O'CONNOR
Head of Project Management and Engineering
National Roads Authority
St Martin's House
Waterloo Road
Dublin 4

ANNEX A: HARMONIC MEAN VISIBILITY

A1 The Harmonic Mean Visibility VISI shall be measured over a minimum length of about 2km in the following manner. Measurements of sight distance shall be taken in both directions at regular intervals (50m for sites of uneven visibility, 100m for sites with good visibility) measured from an eye height of 1.05m to an object height of 1.05m, both above the centre line of the road surface. Sight distance shall be the true sight distance available at any location, taking into account both horizontal and vertical curvature, including any sight distance available across verges and outside the road boundary wherever sight distance is available across embankment slopes or adjoining land, as shown in Figure A1.

A2. Harmonic Mean Visibility is the harmonic mean of individual observations, such that:

$$VISI = \frac{n}{\frac{1}{V_1} + \frac{1}{V_2} + \frac{1}{V_3} \ldots + \frac{1}{V_n}}$$

where:-

n = number of observations
V1 = sight distance at point 1, etc.

A3. For existing roads, an empirical relationship has been derived which provides estimates of VISI given in bendiness and verge width (applicable up to VISI = 720m), i.e.

$$Log_{10}\ VISI = 2.46 + VW/25 - B/400$$

where:

VW = Average width of verge, plus hard shoulder where provided (m, averaged for both sides of the road)

B = Bendiness (degrees per km, measured over a minimum length of about 2 km).

This relationship is valid for most existing roads. However, on long straight roads, or where sight distance is available outside the highway boundary, significant underestimates of VISI will result.

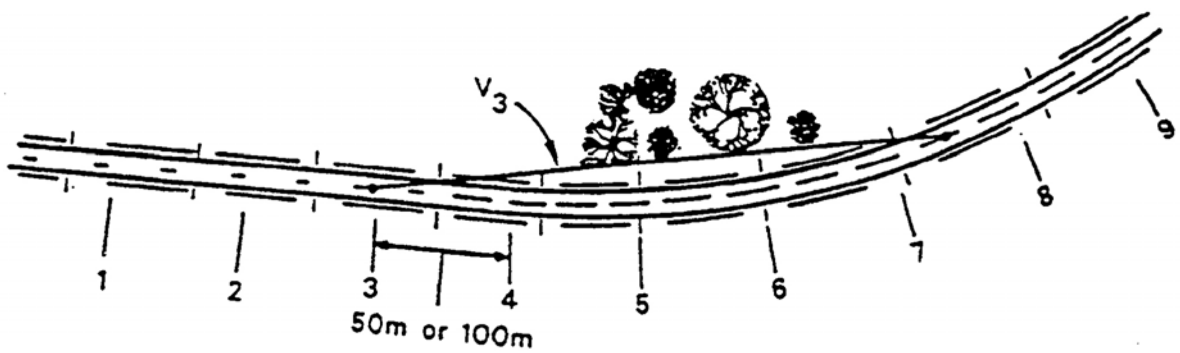


Figure A1: Measurement of Harmonic Mean Visibility

A4. For preliminary route analysis, where detailed measurements of sight distance are not available, the following typical values should be used:

a) On long virtually straight roads, or where the road is predominantly on embankment affording high visibility across embankment slopes or adjoining level land:

VISI = 700m

b) If a new road is designed with continuous overtaking visibility, with large crest K values and wide verges for visibility:

VISI = 500m

c) Where a new road is designed with frequent Overtaking Sections, but with stopping sight distance provision at all sharp curves:

VISI = 300m

d) Where an existing single carriageway contains sharp bends, frequent double continuous line sections, narrow verges etc.:

VISI = 100 to 200m

However, the empirical formula shown in Paragraph A3 can be used if Bendiness is available.

ANNEX B: MONITORING

Introduction.

B0.1 While 2+1 roads are considered appropriate for use on national roads in rural areas it is important to ensure that as much relevant before data is collected on each of these schemes particularly where the scheme being constructed is a retro-fit scheme.

B0.2 The information collected will be used to study the effectiveness of this new road type in Ireland. The following data should be collected and compiled before any work is carried out on the ground.

(a) AADT
(b) junction counts
(c) speed measurements (Retrofit only)
(d) journey time survey
(e) accident details
(f) road geometry (Retrofit Only)
(g) attitude survey

Data Collection

B0.3 AADT. Automatic traffic counters should be used for at least two weeks. These automatic traffic counters should also provide a classification of the different vehicles using the road. Pedestrian and cyclist counts and movements should be carried out at appropriate locations.

B0.4 Junction counts. Full turning movements should be counted at all junctions along the length of the proposed scheme. School Bus, refuse Collection and Public Bus Service schedules should also be examined.

B0.5 Speed measurements. Once an indicative layout for the scheme has been designed it is important to ensure that adequate speed measurements are carried out along the length of the entire scheme. The data should be collected using covert methods preferably pole mounted laser or road surface mats / tubes during both neutral and non-neutral days, as defined in RT 419.. It is recommended that the study be carried out over a minimum of three weekdays.

The following information should then be extracted from the measurements taken.

(a) Individual vehicle speeds and times
(b) Individual vehicle classes
(c) Headway gaps between all vehicles

Speed measurements need to be taken at the following locations along the length of the scheme.

(d) 250m after the start of the full width passing lane.
(e) 250m before the end of the full width passing lane.
(f) 150m after the start of the single lane.
(g) At junctions the measurements should be taken at the end of the right turn lane.

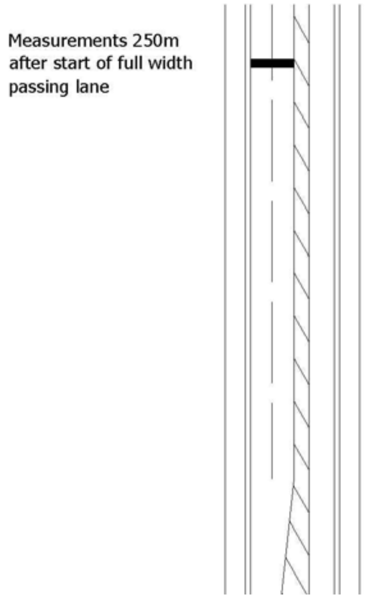


Fig B0.1 250m after the start of the full width passing lane.

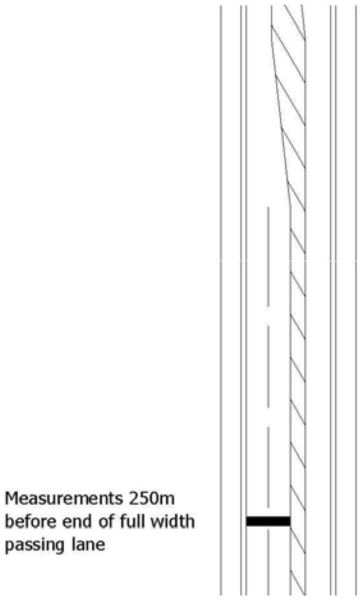


Fig B0.2 250m before the end of the full width passing lane.

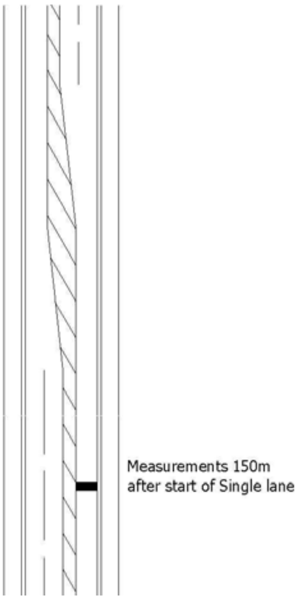


Fig B0.3 150m after the start of the single lane.

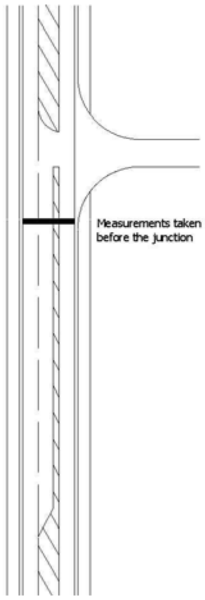


Fig B0.4 At the junction.

B0.6 Journey Time Survey. Normally the survey should reflect average traffic conditions for the year. However in the case of journey time surveys for 2+1 roads, the journey time surveys also need to reflect the journey times during peak and off peak flows.

The surveys should cover neutral and non-neutral days. A number of survey runs should be taken over several days at least two different weekdays in different weeks should be surveyed.

Surveys should not be carried out in adverse weather conditions e.g. heavy rain, flooding, ice or snow.

The survey times are normally from 8 am to 6 pm. Run start times are set in advance and must capture the peak traffic hours on commuter routes. These should be at regular intervals throughout the day. The maximum interval is 30 minutes giving two runs per hour; 1 going and 1 returning.

At sites where there is occasional severe disruption (e.g. Queues due to slow moving vehicles), which results in a significant increase in journey time, it could happen that such events are missed in a survey. In that case, a statement to that effect should be attached to the journey time data giving details of the delay experience and frequency of such events.

The overall number of runs needed to give statistical confidence in the results are listed in table B0.6.

The column and row which best matches site conditions are identified. The left column applies where speed is somewhat restricted by traffic conditions and the right column where it is severely restricted. Other traffic conditions are interpolated in between. The appropriate row is that which agrees best with the reduction in journey time expected from the road improvement.

Traffic Conditions	Speed restricted somewhat by traffic flow			Unstable flow; stoppages for periods		
Coefficient of variation s/x_2 **		0.1	0.2	0.3	0.4	0.5
Expected Improvement percent ***	5	23	86	195	-	-
	10	7	23	51	87	136
	20	3	7	13	23	35
	30	3	4	7	11	16
	50	3	3	3	5	7

Table B0.6

* Gives confidence at the 95 percent level when comparing sample means.
** S = standard deviation of journey times (before improvement)
x_1 = mean of journey before road improvement
x_2 = mean of journey after road improvement
*** Anticipated reduction in journey time to follow opening of the new road, percent; $100(x_1-x_2)/x_1$

The required number of runs is read off and compared with the figure that is envisaged. If necessary, the plan is expanded to meet the requirement of Table B0.6. Frequently this will not be necessary, but it should also be checked. To avoid driver bias, at least two drivers should be involved.

Example:

The proposal is to by-pass a village with a new road. The subjective assessment of average traffic conditions at the village is midway between slight restriction and unstable with stoppages i.e. "Coefficient of variation s/x_2 is 0.3". Journey time is expected to be reduced by 20% when the new road is built.

It is recommended that the maximum journey time interval is 30 minutes. That is two runs per hour (1 going, 1 returning). That is 20 runs per day (10 hours: 8am to 6 pm). For a two day survey the total number of runs envisaged is 2 * 20 = 40.

Identify the above mentioned traffic conditions and expected improvement assumptions in table 10.6. A minimum figure of 13 runs is indicated. As this is less than the planned 40 runs, the survey as envisaged is adequate.

Procedure:

Drivers should wait at designated waiting points. They should commence runs as soon as possible after start times indicated on the journey time sheets. Before starting, the actual start time (hour:minute) should be entered on the sheet. The driver should follow the next vehicle(s) to arrive. Then the floating car should be driven at the same average speed as the rest of the traffic. Observers will confirm this by ensuring that the number of vehicles overtaken, on average equals the number overtaking the observer vehicle. Speed limits should be observed.
The time taken to travel between trip end points is read from the stopwatch (hour:minute:second) and entered at the end of the run. If necessary, the driver should be assisted by an observer.
If runs are missed, these should be done at the same time on another day.

Journey Time Summary Sheet

Project:	Date Weekday
Trip End A:	Survey Before [] or After []
Trip End B:	Road Improvement? Tick one.
Weather:	Drivers Signature :
End A: Easting	Northing
End B: Easting	Northing

A to B			B to A		
Nominal Start Time hh:mm	Actual Start Time hh:mm	Duration hh:mm:ss	Nominal Start Time hh:mm	Actual Start Time hh:mm	Duration hh:mm:ss

		A to B	B to A	Overall
Standard deviation	s			
Mean	x			
Coefficient of variation	s/x			

Additional information can be found in "RT 419, Journey Time Measurement, for the assessment of major road projects".

B0.7 Accident Details. A full accident study looking at the accidents that have occurred along the length of the scheme. Further analysis should be carried out at individual junctions along the length of the scheme. When analysing the accident data, the following information needs to be analysed.

(h) Accident Types
(i) Accidents by Year, Week, and Date
(j) Accidents by lighting condition
(k) Accidents by weather condition
(l) Accidents by surface condition
(m) Accidents by road character
(n) Accidents by primary collision type
(o) Junction type
(p) Junction control

This information should be broken into two main sections mainline related accidents, and junction related accidents.

When gathering this information it is important to ensure that the paper copies of the CT68's / PC16's are used when gathering this information.

B0.8 Road Geometry. It is important to ensure that a full study is undertaken of the existing road geometry. The following information needs to be documented, Vertical and horizontal geometry, Junction numbers, types and layouts.

B0.9 Attitude Survey, it is important to ensure a full attitude survey is carried out. The survey should aim to survey either 3% of the AADT or 250 drivers whichever is the greater. The survey should be carried out on the existing road.

B10. Monitoring Time Periods. All surveys must be carried out prior to the scheme starting construction, the following surveys must be repeated at 6 months, 1 year and 3 years after the scheme have been completed.

(q) AADT
(r) junction counts
(s) speed measurements
(t) journey time survey
(u) accident details
(v) attitude survey

B11. Monitoring Report. A monitoring report containing this information must be prepared prior to the issuing of tender documentation.

2+1 Questionnaire and Attitude Survey

2+1 QUESTIONNAIRE AND ATTITUDE SURVEY

1.1 Type of Vehicle	**What type of vehicle do you drive**	
1.2 Age	**1 = <40 or 2 = >40**	
1.3 Sex	**Male or Female**	
Questions		Question
1.4 Type of Licence	**Full or Provisional**	
1.5 Driving Distance / Year	**Approx. how many miles do you drive per year**	
1.6 Residence / Location	**Where do you live (location, town)**	
1.7 Reason for Driving	**What is the purpose of the journey**	
1.8 Frequency of Driving	**How often do you drive this section of the road**	
Best Lane Layout		Road Type
2.1 Overtaking	**Which type do you prefer for overtaking**	
2.2 Distance	**Which type makes the driver keep the safest distance between vehicles**	
2.3 Driving	**Which type are you more comfortable with.**	
Traffic at different layouts		Layouts
3.1 Speeding	**Which type do you think people drive fastest on**	
3.2 Overtaking	**Which type do you think allows the most safe overtaking on**	
3.3 Risk	**Which type do you think has the most incidents**	
3.3 Accidents	**Which type do you think could have the more serious incidents**	
4.1 Best Layout	**In summary which road type do you prefer**	

Vehicle Type
1. Motor Car
2. Van or Light Goods Vehicle
3. Truck (Rigid)
4. Artic
5. Motor Cycle

Sex
1. Man
2. Women

Licence
1. Full
2. Provisional

Reason for Driving
1. To / From Work
2. To / From School / College
3. In Course of Work
4. Shopping
5. Leisure / Tourist
6. Other

Frequency of Driving
1. Often
2. Seldom
3. 1st. Time

Road Type
1. Single Carriageway
2. 2+1 Carriageway
3. No Difference
4. Don't Know

Any Comments

ANNEX C : MAINTENANCE OF 2+1 SCHEMES.

Prior to scheme opening
(Fire & Rescue Staff training)

Training on removing and repairing safety barrier to be given by contractor prior to completion of project. It is suggested that a 50m length of safety barrier be erected offsite for training purposes. (The erection of same to be included for in the contract). Attendees to include Fire Officers, Gardai, Ambulance Officers, Local Authority personnel. NRA have produced a brochure that can be handed out at this training session.

Keep stock of spares …. Usually 5% of the original contract. Include for same in original contract.

Ensure that equipment for re-tensioning the safety barrier is provided as part of the contract and that training in its use is done at Fire & Rescue staff training session.

For first month after scheme opening:
- Drive the full route in early morning (7 to 8am) to pick up any damage to posts during the night.
- Drive the route a total of 3 times a day for first month to pick out incidents.

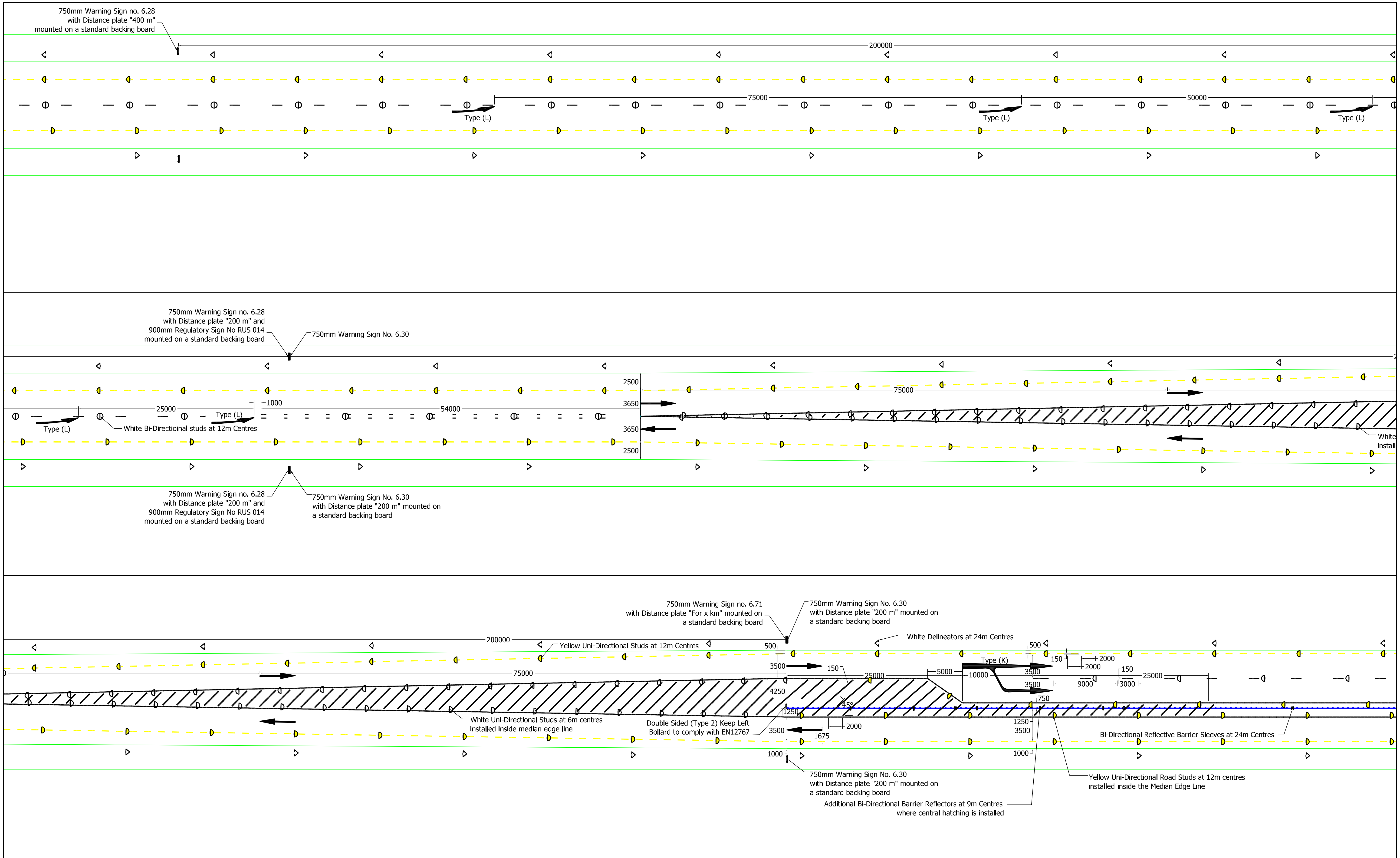
After 1st month (review, but probably as follows)
- Drive the full route in early morning (7 to 8am) to pick up any damage to posts during the night.
- Drive the route a total of 2 times a day for first month to pick out incidents.

Response times to various incidents
- If post bent but in median, replace within one week
- If up to 2 Posts on Carriageway, remove immediately and replace within 3 days.
- If more than 2 Posts on Carriageway, remove immediately and replace within 12 hrs.

Incident Detection

[TEXT TO FOLLOW]

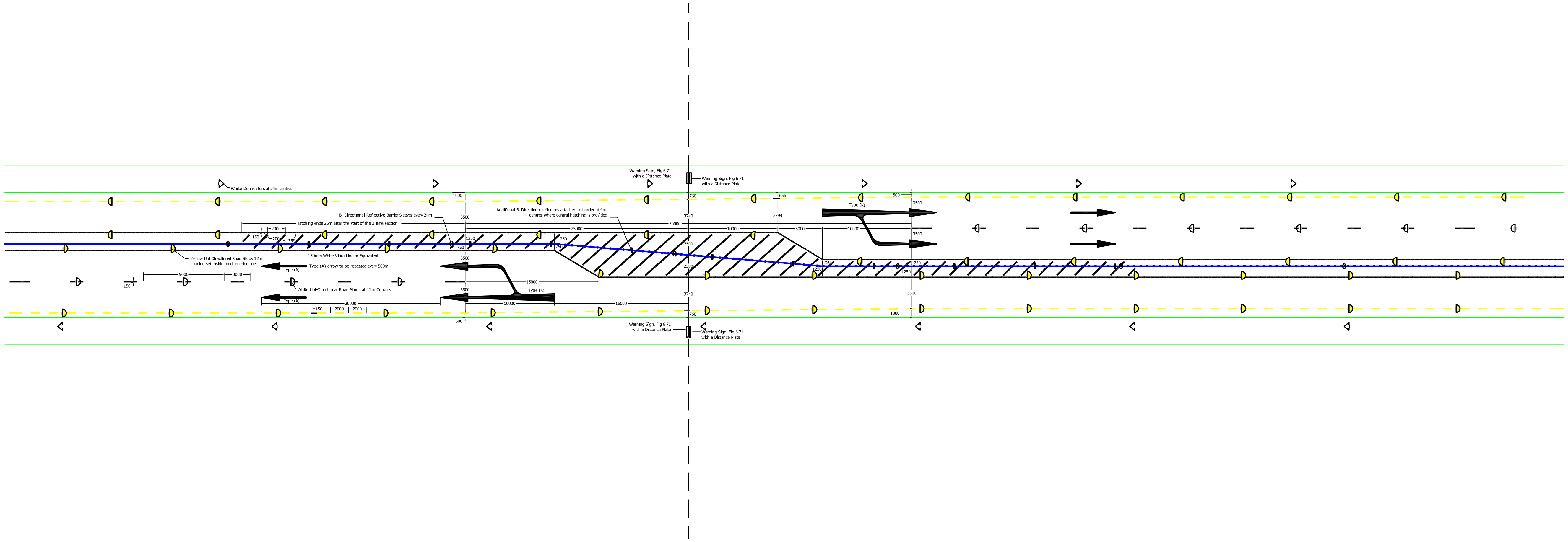
ANNEX D : RECOMMENDED LAYOUTS FOR 2+1 JUNCTIONS.



National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts

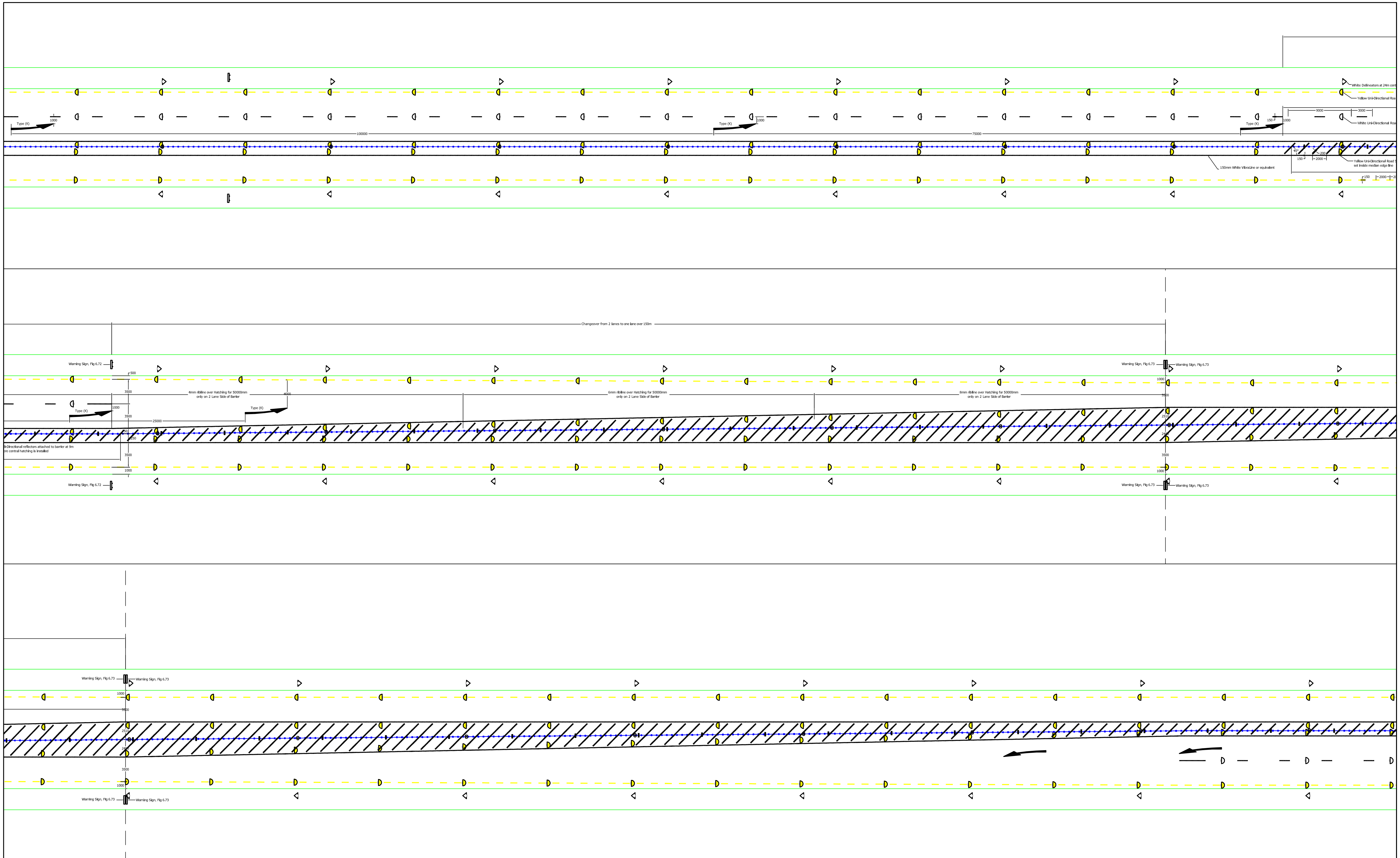
Section	2+1 Details	Scale N.T.S
Title	Lining at Start of 2+1	Drg. No. 2+1/RL/001



NRA
National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts

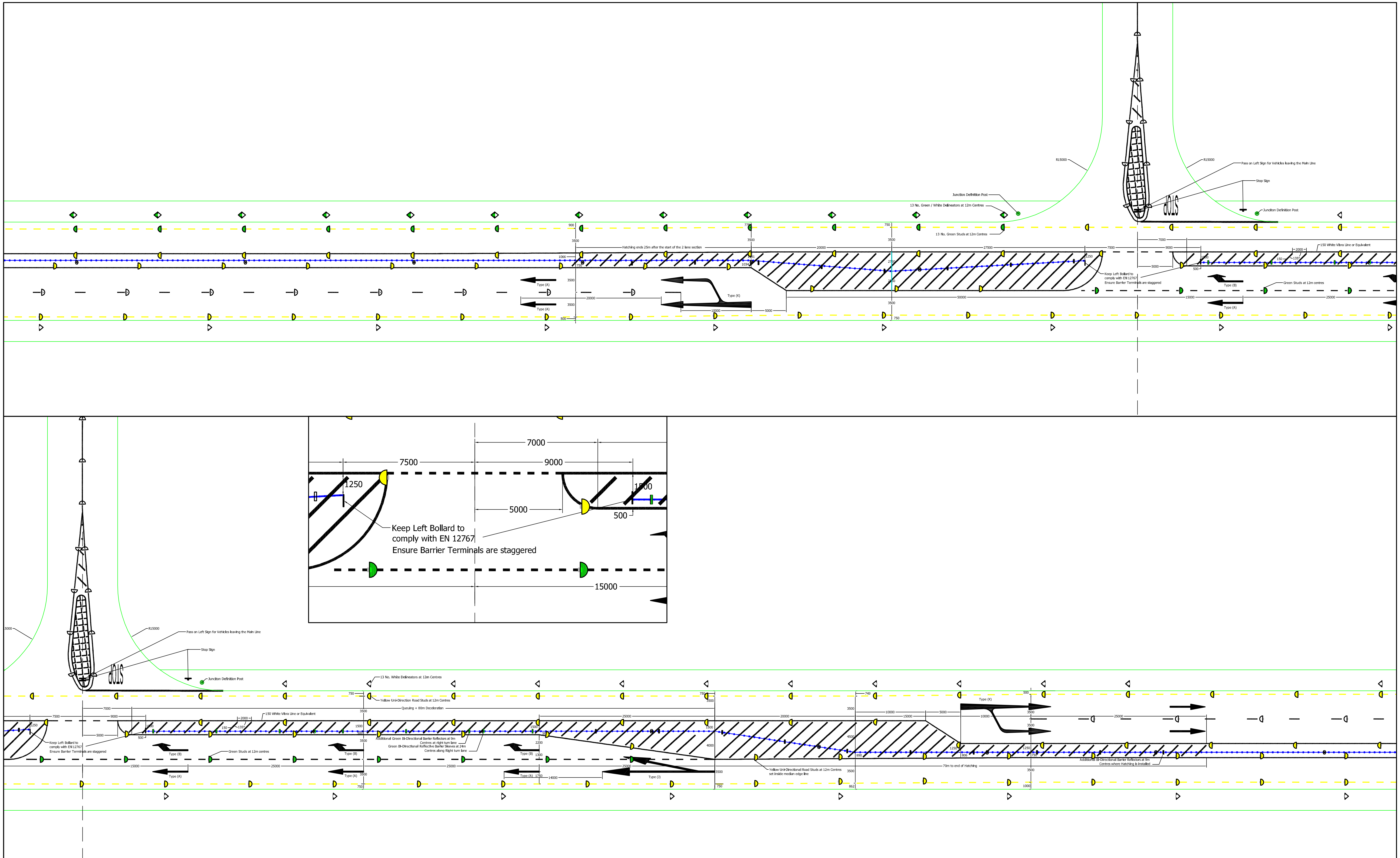
Section	2+1 Details	Scale	N.T.S
Title	Non Critical Changeover	Drg. No.	2+1/RL/002



NRA National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts

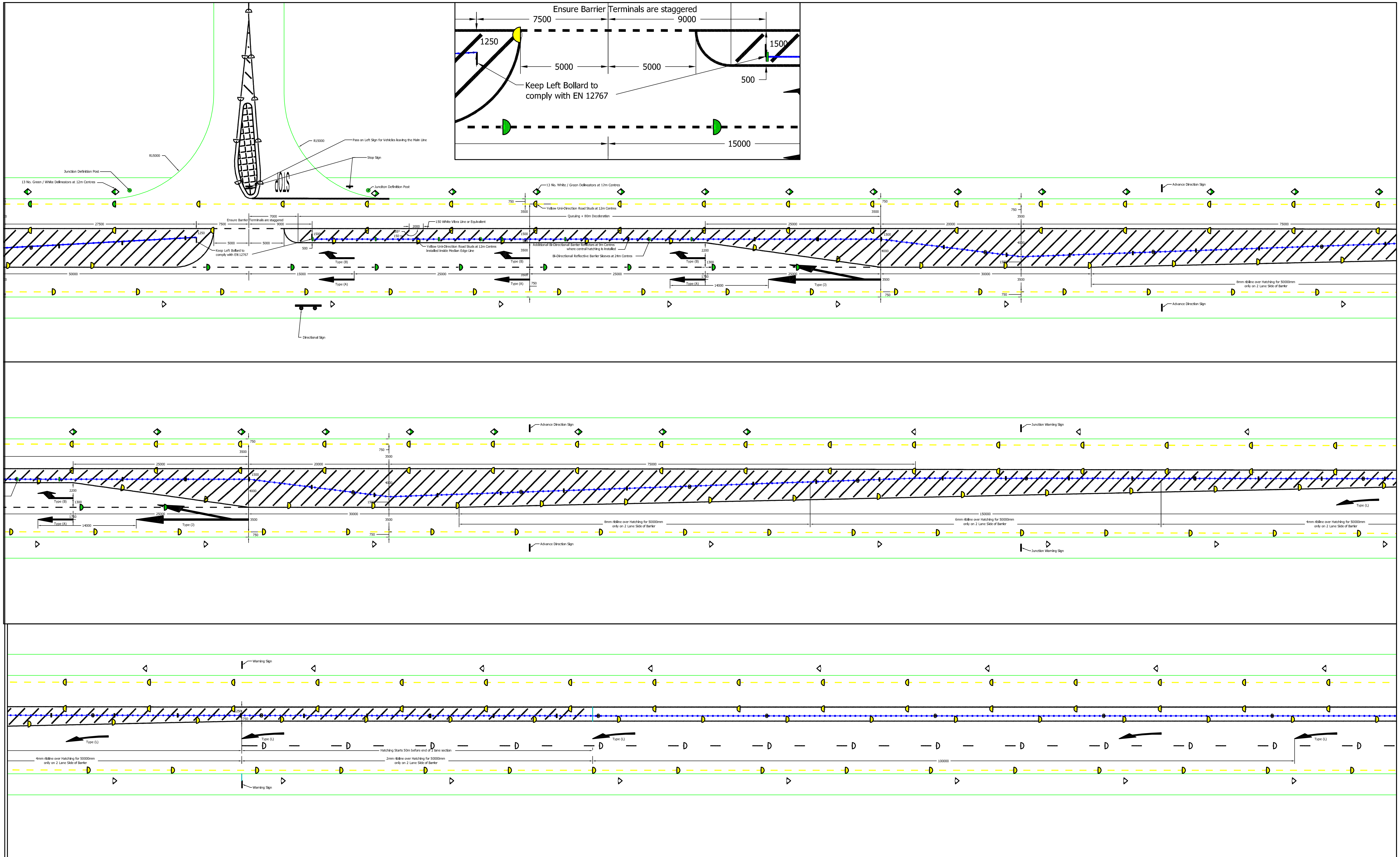
Section	2+1 Details	Scale N.T.S
Title	Critical Changeover	Drg. No. 2+1/RL/003



National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts

Section	2+1 Details	Scale N.T.S
Title	Single Lane to Right Turn	Drg. No. 2+1/RL/004



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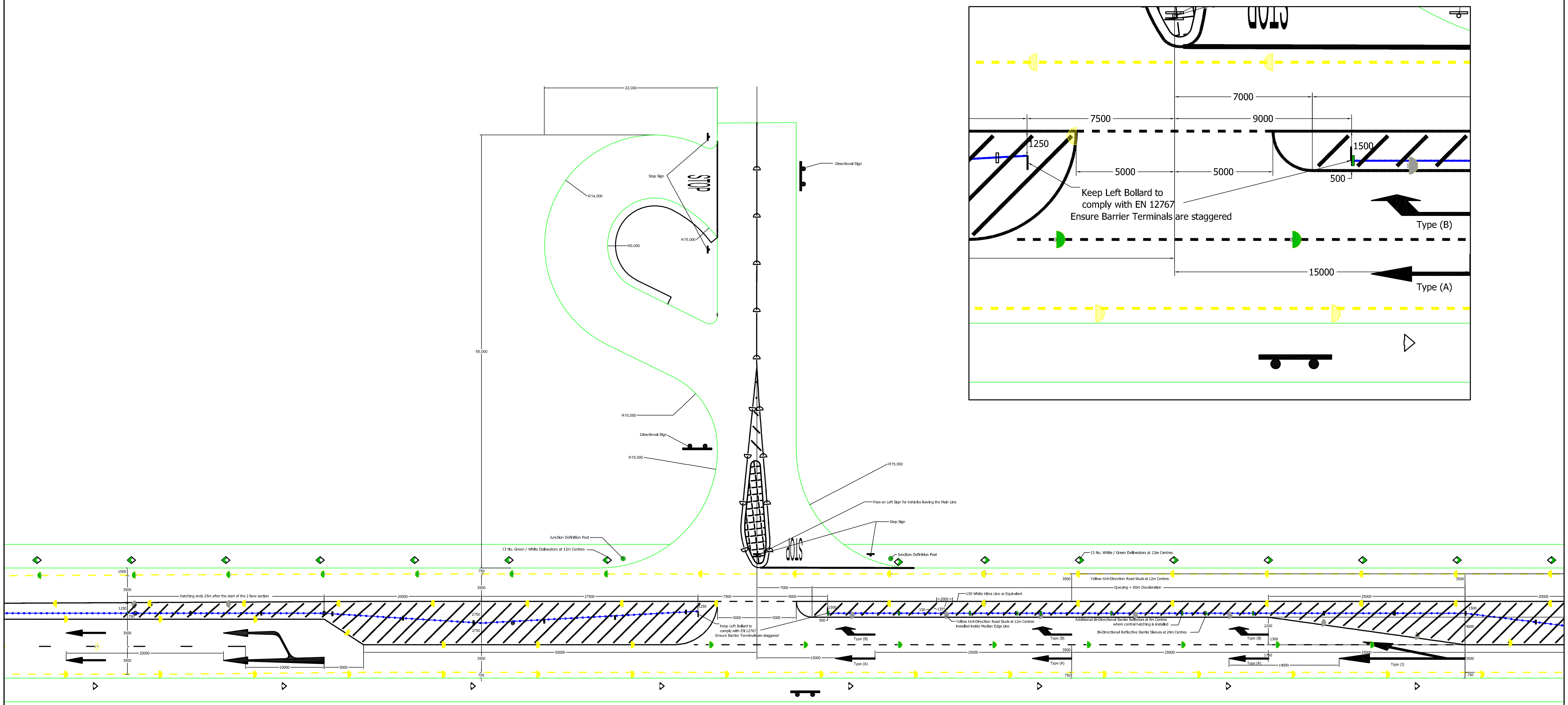
Recommended Layouts

Section	2+1 Details	Scale N.T.S
Title	2 Lane to Right Turn	Drg. No. 2+1/RL/005



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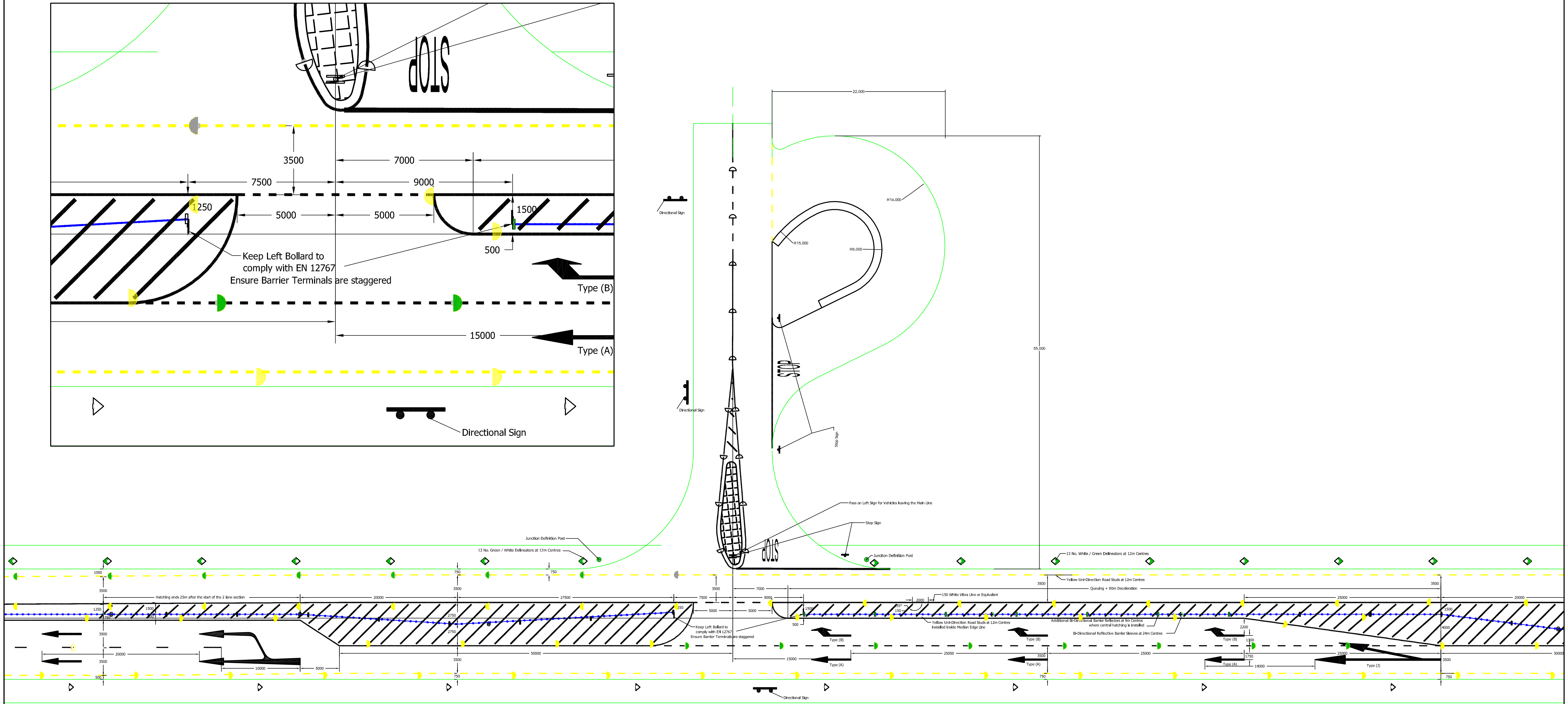
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts		
Section	2+1 Details	Scale N.T.S
Title	Right Turn with U-Turn to Left	Drg. No. 2+1/RL/006



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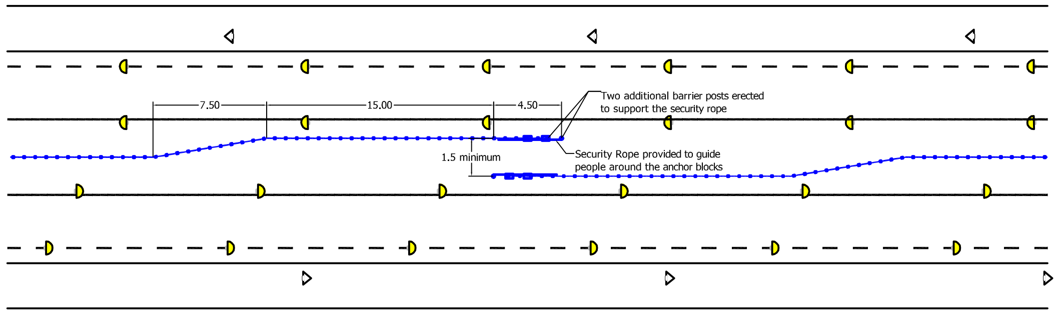
Recommended Layouts

Section	2+1 Details	Scale N.T.S
Title	Right Turn with U-Turn to Right	Drg. No. 2+1/RL/007



National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts		
Section	2+1 Details	Scale N.T.S
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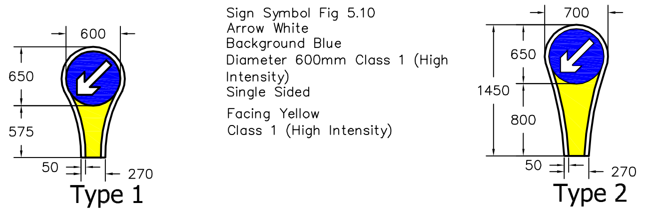
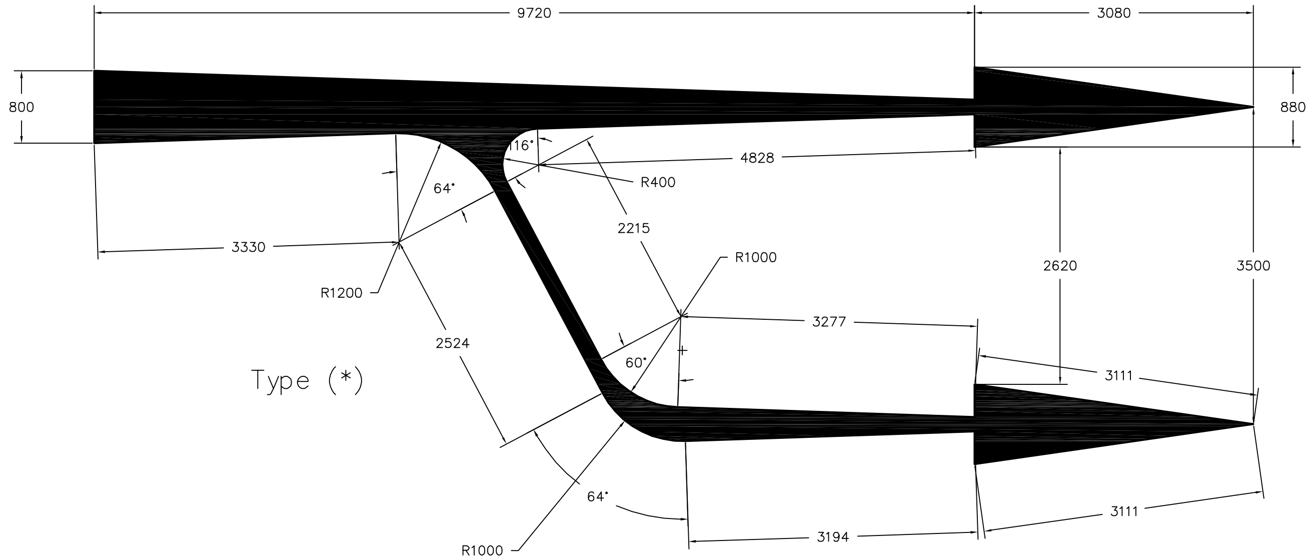
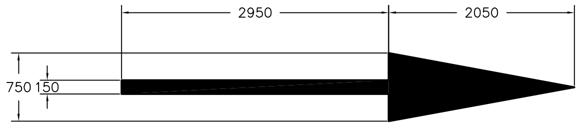
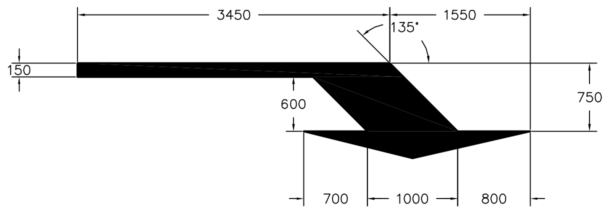




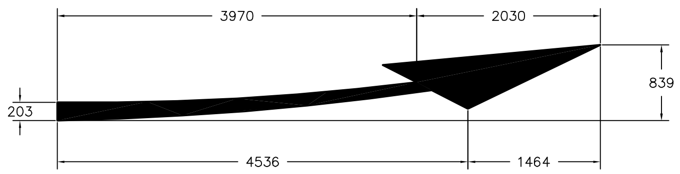
Type (K)



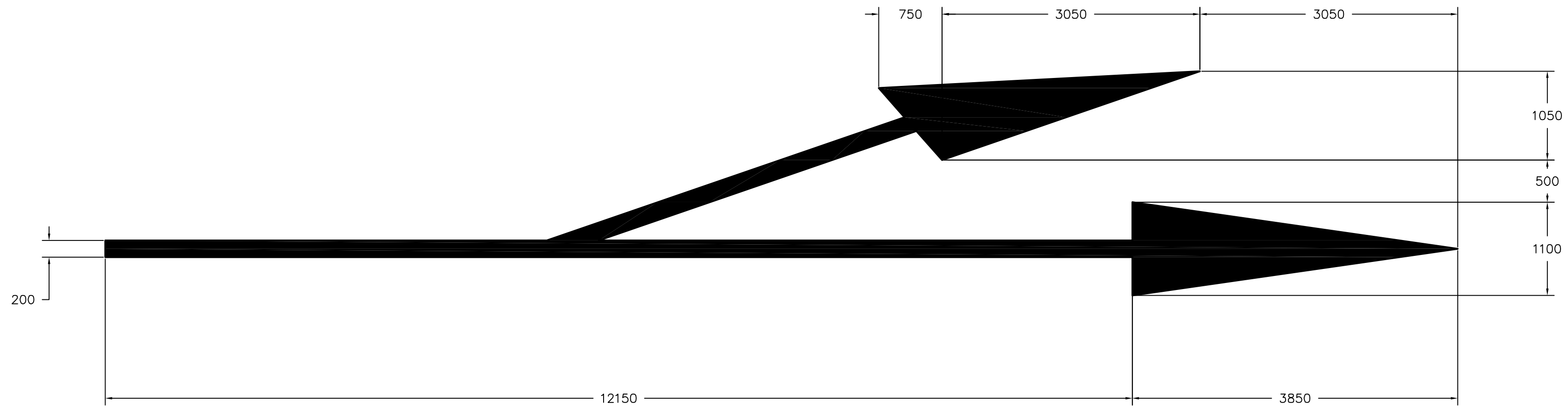
Type (A)



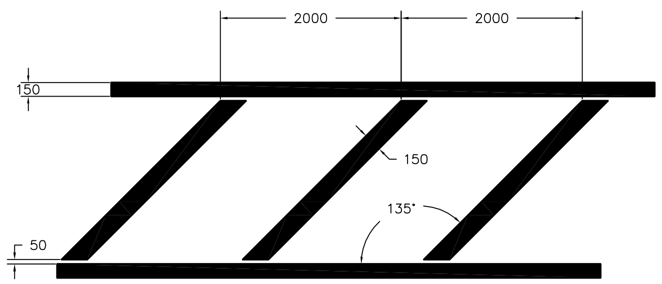
Type (B)



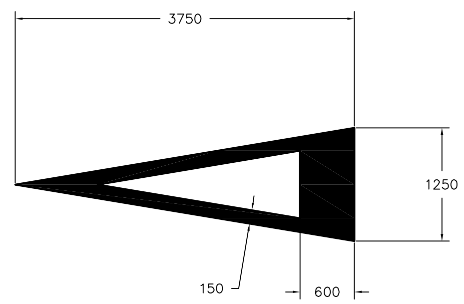
Type (L)



Type (J)



Hatch Detail



National Roads Authority
An tÚdarás um Bóithre Náisiúnta

Recommended Layouts		
Section	2+1 Details	Scale N.T.S
Title	Arrow Details	Drg. No. 2+1/RL/100